

Final Report

FUNCTIONAL SERVICING and STORMWATER MANAGEMENT REPORT

1 Rosetta Street



Prepared for 1 Rosetta Street (Halton Hills) GP Limited
by IBI Group
May 12, 2022

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1 Introduction

1.1 Background

IBI Group has been retained by 1 Rosetta Street (Halton Hills) GP Limited to prepare a Functional Servicing Report (FSR) for a proposed multi-use residential building at 1 Rosetta Street in Georgetown (Town of Halton Hills).

The purpose of this report is to development a municipal site servicing strategy (stormwater, sanitary discharge, and water supply). More specifically, the report will present the following:

- Calculate allowable and proposed runoff rates for the development;
- Evaluate suitable methods for attenuation and treatment of stormwater runoff;
- Develop on-site control measures and examine theoretical performance;
- Identify sanitary servicing opportunities and constraints and evaluate the capacity of the receiving municipal sewer.
- Identify water servicing opportunities and constraints, calculate the proposed domestic water and firefighting supply needs; and evaluate the capacity of the municipal infrastructure.

The following documents have been obtained from various sources:

- Halton Region plan and profile drawings for River Drive and John Street;
- Topographic Survey prepared by J. D. Barnes Ltd., dated July 2, 2020; and,
- Architectural plans and site statistics prepared by Icon Architects.

1.2 Existing Site Description

Under the existing conditions, the 1.343-ha site consists of three parcels and is currently used as single family residential and industrial warehouse. The properties will be merged as part of this application. Please see **Figure 1** following the report for an aerial view of the site.

1.3 Site Proposal

The proposed development includes the construction of two (2) twelve-storey residential building and one (1) eight-storey residential building, including proposed underground parking structure, surface level parking, drive aisles and landscaped areas. A preliminary concept plan is provided in **Appendix A**.

2 Terms of Reference and Methodology

2.1 Terms of Reference

The terms of reference used for the scope of this report have been based on the Halton Region Water and Wastewater Linear Design manual, dated April 2019.

2.2 Methodology: Water Supply

The domestic water usage will be calculated based on the following Region of Halton and Ontario Building Code design criteria:

Table 2.1 Water Design Parameters

Population Density		Peaking Factors		
		Land Use	Peak Hour	Max Day
1 Bedroom Unit	1.328 people/unit	Residential	4.00	2.25
2 Bedroom Unit	1.724 people/unit			
Average Daily Demand	275 L/person/day			

Pressure and flow testing to determine the adequacy of the existing watermain to support the development with fire suppression in accordance with the Fire Underwriters Survey (FUS) Guidelines will be discussed in the subsequent sections.

2.3 Methodology: Sanitary Discharge

Pre- and post-development peak sewer flows will be calculated based on the following Halton Region design criteria:

Table 2.2 Sanitary Design Parameters

Design Flows		Population Densities	
275 L/c/day	Proposed Residential	1.328 people / unit	1 Bedroom Units
0.286 L/s/ha	Infiltration Allowance	1.724 people / unit	2 Bedroom Units
Peaking Factor	Harmon Equation		

Based on the calculated peak flows, the adequacy of the existing infrastructure to support the proposed development will be discussed.

2.4 Methodology: Stormwater Management

As identified in the pre-consultation for the development, stormwater management will be required to be reviewed at the Zoning Application Stage.

Quantity Control

Post-development flows for all storm events are to be attenuated to the corresponding pre-development levels.

Quality Control

Long-term average removal of 80% of the total suspended solids (TSS) on an annual loading basis must be achieved.

3 Water Supply System

3.1 Existing Water Infrastructure

Per the Region's record information, the following water infrastructure is available in the vicinity of the site:

- 150mm dia. watermain on St. Michaels Street
- 150mm dia. watermain on Caroline Street
- 150mm dia. watermain on Rosetta Street
- 300mm dia. watermain on Rosetta Street
- 300mm dia. watermain on River Drive

Additionally, a variety of fire hydrants are located in proximity of the site:

- Fire Hydrant on River Drive at St. Michael Street
- Fire Hydrant on River Drive (south east corner of the 1 Rosetta St property)
- Fire Hydrant on River Drive (south west corner of the 2 Rosetta St property)
- Fire Hydrant on River Drive at Rosetta Street
- Fire Hydrant on Rosetta Street at Caroline Street

The existing industrial warehouse is serviced via two services extending from the 300mm dia. Watermain along River Drive. The water services are 150mm and 200mm in diameter. The existing residential lots on St Michael Street are serviced via individual service connections to the existing 150mm dia. watermain on St. Michaels Street.

3.2 Domestic Water Supply Demands

Using the criteria set in **Section 2.4** and the site statistics provided by the architect, the Average Day Demand (ADD), Peak Hour Demand (PHD), and Max Day Demand (MDD) have been calculated based on the number of units, as shown in **Table 3.1**.

Table 3.1 Domestic Water Demands

Building	Number of Units	Population	ADD (L/s)	PHD (L/s)	MDD (L/s)
Residential – 1 Bedroom	490	651	2.07	8.28	4.66
Residential – 2+ Bedrooms	150	259	0.82	3.29	1.85
Total	640	909	2.89	11.58	6.51

The domestic supply line for the building will be designed based on PHD while maintaining a minimum available pressure of 40 psi (275 kPa) at the face of the building. Please see **Appendix B** for the detailed calculations.

3.3 Fire Supply Demands

The recommended fire flow demand for the subject site has been calculated using the design criteria outlined in the Water Supply for Public Fire Protection Manual, 1999 by the Fire Underwriters Survey (FUS).

As the building will be constructed using fire resistive materials, the effective floor area is taken as the largest floor area plus 25 % of the two adjacent floors.

- Effective Floor Area = Largest Floor Area + 25% (two adjoining floors)
- Effective Floor Area = 3,148 m² + 25% (3,148 m² + 3,148 m²)
- Effective Floor Area = 4,722 m²

The corresponding floor area and FUS factors will be applied as follows:

Table 3.2 Fire Underwriters Survey Factors

Construction Coefficient	Building Occupancy	Sprinkler Adjustment	Proximity Factor
0.6 (resistive)	- 15 % (limited)	- 30 %	+ 30 %

Using the effective floor area for each building and the appropriate FUS factors, the required fire flow for each building is calculated as follows:

Table 3.3 Fire Demand Calculations

Fire Flow (F) Calculation	Applying FUS factors	Adjusted Fire Flow	Total Demand (TD)
$F = 220 \cdot 0.6 \sqrt{\text{Area}}$	$F_1 = F \cdot 0.85 = 7,650 \text{ L/min}$	Fire Flow = $F_1 - F_2 + F_3$	TD = FF + MDD
$F = 220 \cdot 0.6 \sqrt{4,722 \text{ m}^2}$	$F_2 = F_1 \cdot 0.30 = 2,295 \text{ L/min}$	FF = 8,000 L/min (rnd'd)	TD = 133.3 L/s + 11.6 L/s
$F = 9,000 \text{ L/min (rnd'd)}$	$F_3 = F_1 \cdot 0.30 = 2,295 \text{ L/min}$	FF = 133.3 L/s	TD = 139.8 L/s

The fire supply line for the building will be designed based on Total Demand (Fire Flow + MDD) while maintaining a minimum available pressure of 20 psi (140 kPa) at the face of the building. Please see **Appendix E** for the detailed calculations.

3.4 System Pressure Under Normal Operation

As previously mentioned, the domestic service shall be sized to convey domestic demands under normal system operating conditions (PHD) while maintaining a minimum available pressure of 40 psi (275 kPa). The residual pressure at the building is calculated by first interpolating the PHD residual pressure within the existing watermain, and then subtracting head losses within the system using the Hazen-Williams formula. The following table summarizes the residual pressure for the proposed domestic service:

Table 3.4 Residual Pressure under PHD Conditions

Flow Conditions	PHD (L/s)	Domestic Service (mm)	Residual Pressure @ Main		Residual Pressure @ Bldg.	
			(psi)	(kPa)	(psi)	(kPa)
PHD	11.58	200	64.9	447	64.2	442

As shown above, there is no appreciable head loss within the system, and the residual pressure at the building face is above the minimum acceptable pressure of 40 psi (275 kPa) under PHD conditions. Please see **Appendix B** for the detailed design calculations.

3.5 System Pressure Under Fire Flow

As previously mentioned, the fire service shall be sized to convey the total fire demand (Fire + MDD) while maintaining a minimum available pressure of 20 psi (140 kPa). The residual pressure at the building is calculated by first interpolating the residual pressure within the existing watermain, and then subtracting head losses within the system using the Hazen-Williams formula. The following table summarizes the residual pressure for the proposed fire service:

Table 3.5 Residual Pressure under Fire + MDD Conditions

Flow Conditions	FF+MDD (L/s)	Fire Service (mm)	Residual Pressure @ Main		Residual Pressure @ Bldg.	
			(psi)	(psi)	(psi)	(kPa)
FF+MDD	139.8	200	53.2	367	38.1	263

As shown above, the residual pressure at the building face for the fire service is above the minimum acceptable pressure of 20 psi (140 kPa) under fire demand conditions (Fire + MDD). Please see **Appendix B** for the detailed design calculations.

3.6 Water Service Connection

The existing 150 mm and 200 mm water services will be removed, with a new 200 mm fire service and a 150 mm domestic service proposed to service the development.

3.7 Hydrant Coverage

The hydrants along the north side of River Drive along the 1 Rosetta Street property will be relocated and/or decommissioned. As previously mentioned, the building will be sprinklered, therefore, a private hydrant is proposed to be included south of the above ground parking in the centre of the subject site and shall be placed within 45 m of the Siamese connections to satisfy OBC requirements.

Please see drawing **SS-01** for the location of all existing and proposed water infrastructure.

4 Sanitary Drainage System

4.1 Existing Sanitary Drainage System

Per the City's record information, local sanitary infrastructure consists of:

- a 200mm dia. sanitary sewer on St. Michaels Street;
- a 200mm dia. sanitary sewer on Caroline Street; and
- a 250 mm dia. sanitary sewer on Rosetta Street.

Existing sanitary infrastructure is shown on the engineering drawing **SS-01** which can be found in **Appendix E** for reference.

4.2 Pre-Development Sanitary Design Flow

Under existing conditions, the site houses an industrial paper mill and a couple of residential dwellings. Therefore, taking into account infiltration, the pre-development peak sanitary flow are summarized in the table below:

Table 4.1 Pre-Development Sanitary Flows

Land Use	Area (ha)	Density	Population	K _{av}	Peaking Factor	Sewage/Industrial Flow (L/s)	Infiltration Flow (L/s)	Total Flow (L/s)
Industrial	1.3492	125 pp/ha	162	0.81	3.39	0.067	0.386	0.45
Residential	0.0865	55 pp/ha	5	0.81	3.61	0.015	0.025	0.04
Total								0.49

4.3 Post-Development Sanitary Design Flow

Based on the criteria set in **Section 2.3**, the corresponding post-development sanitary sewer flows are summarized below:

Table 4.2 Post-Development Sanitary Flows

Land Use	Area (ha) Number of Units	Density	Population	K _{av}	Peaking Factor	Sanitary Flow (L/s)
Infiltration	1.4356			1	3.83	0.411
1 Bedroom	490 Units	1.328 pp/unit	651	1	3.83	7.936
2+ Bedrooms	150 Units	1.724 pp/unit	259	1	3.83	3.157
Total						11.504

As shown above, the post-development sanitary sewer flow is calculated to be 11.504 L/s. Please refer to the detailed design sheet which can be found in **Appendix C**.

4.4 Sanitary Service Connection

It is proposed that a new 250 mm sanitary service at a 1.0% slope be installed from the control manhole at the property line to a new municipal manhole within the River Drive and Rosetta Street intersection. The following table illustrates the peak flow and corresponding capacity of the proposed sanitary service and sewer:

Table 4.3 Sanitary Service Performance

From	To	Pipe Size (mm)	Pipe Slope	Peak Flow (L/s)	Capacity (L/s)	Percent of Full Flow
Cntrl.MH	Existing Manhole	250	1.0 %	11.504	62.0	18.5%

As shown above, the proposed sanitary service and sewer will easily convey the post-development peak sanitary flow while operating at 18.5% or less of full flow capacity. Please see the detailed design sheet which can be found in **Appendix C**, and Drawing **SS-01** which can be found in **Appendix E**.

4.5 Down Stream Analysis

The Sanitary Capacity Review completed by TMIG (February 2022) indicates the sanitary peak flows are 11.1 L/s based on the proposed population density and the Region's sanitary peak flow per capita. Based on the review of the existing system, the sanitary sewer on River Drive has sufficient capacity for the flows anticipated from this development. A memo detailing the Sanitary Capacity Review is included in **Appendix C**.

5 Stormwater Management

5.1 Pre-Development Conditions

Per the City's record information, local storm infrastructure consists of:

- 450mm dia. storm sewer on River Drive (east)
- 250mm dia. storm sewer at the corner of Rosetta Street and Caroline Street
- 300mm dia. storm sewer on River Drive (west)

The site is largely occupied by the existing industrial building and accompanying parking lot.

5.2 Grading

Under pre-development conditions the site topography falls from north to south (Caroline Street to River Drive). The existing loading dock houses a localized low point drained by an existing catch basin, with additional catch basins located to the northeast of the site on Rosetta Street, at the intersection of River Drive and Rosetta Street as well as River Drive and St Michael Street. Drainage in the right-of-way is directed to existing swales along the roadside where it is collected by this storm infrastructure.

The proposed grades will match current drainage patterns and grades will be maintained along property lines to the extent practical. The proposed site plan features a 'woonerf', an open style street, with no curb for ease of movement.

Emergency overland flow route in excess of a 100-year storm event will be directed along the internal roadway to the municipal right-of-way.

5.3 Quantity Control

Under existing conditions, the subject site has a runoff coefficient of 0.85. The proposed development will remove the existing structure and much of the paved area, resulting in a reduction of imperviousness and a runoff coefficient of 0.62 under proposed conditions.

This reduction in imperviousness will result in a reduction of peak, resulting in all post-development peak flows remaining less than or equal to predevelopment peak flows. Impervious areas of pre- and post-development conditions are summarized below.

Table 5.1 Pre-Development Site Imperviousness

	Area (m ³)	Runoff Coefficient	Contributing Coefficient
Conventional Roof	8,269	0.9	0.55
Landscaped Area	1,002	0.25	0.02
Impervious Area	4,301	0.9	0.28
Total	13,572		0.85

Table 5.2 Post-Development Site Imperviousness

	Area (m ³)	Runoff Coefficient	Contributing Coefficient
Conventional Roof	6,271	0.9	0.42
Landscaped Area	4,005	0.25	0.07
Permeable Pavers	3,296	0.55	0.13
Total	13,572		0.62

As demonstrated in the tables above the proposed development will reduce the overall site imperviousness. This will in turn reduce peak flows from the site and mitigate the need for stormwater quantity controls.

5.4 Quality Control

As previously mentioned, 80% TSS removal is required to provide enhanced water cleansing to the site. Parking and drive aisle areas are proposed to be paved with permeable pavers to provide enhanced cleanings to stormwater flows.

5.5 Storm Sewer Connection

A storm sewer network is proposed to be included through the site drive aisle, catch basins will be placed at low points to collect run off and covey flows to the manhole within the River Drive boulevard. Please refer to the detailed design calculations which can be found in **Appendix D**, and the design **Drawing SS-01** which can be found in **Appendix E**.

5.6 Emergency Overflow

Overland flow from the proposed development will continue to be directed to River Drive, St. Michaels Street, Rosetta Street, and Caroline Street.

6 Conclusions and Recommendations

Storm Sewer and Stormwater Management

The proposed development will see a reduction in the imperviousness of the site, this in turn will result in a reduction of peak flows, eliminating the need for stormwater attenuation.

By incorporating inherently clean rooftop, landscape, and pavers, the site will meet the target for quality control.

Sanitary Sewers

As the site represents a manageable increase in sanitary flow, the proposed development can proceed without improvements to the municipal sewer system.

Water Supply

The existing municipal water supply has sufficient capacity to support the proposed fire and domestic water demands without improvements to the system.

Summary

In summary, it can be concluded that the Zoning By- Law Amendment can be supported for the proposed development from both municipal servicing and stormwater management perspectives.

Should you have any questions, please do not hesitate to contact the undersigned.

Respectfully Submitted,

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Figure 1

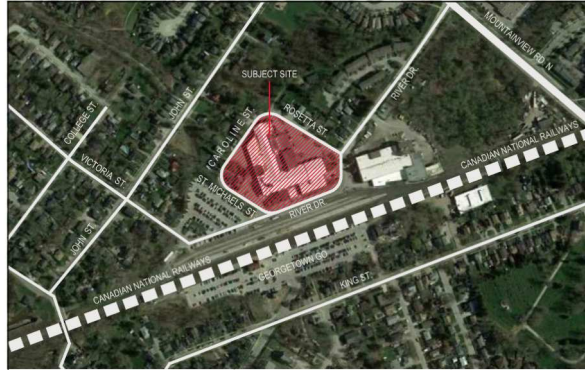
Aerial Plan



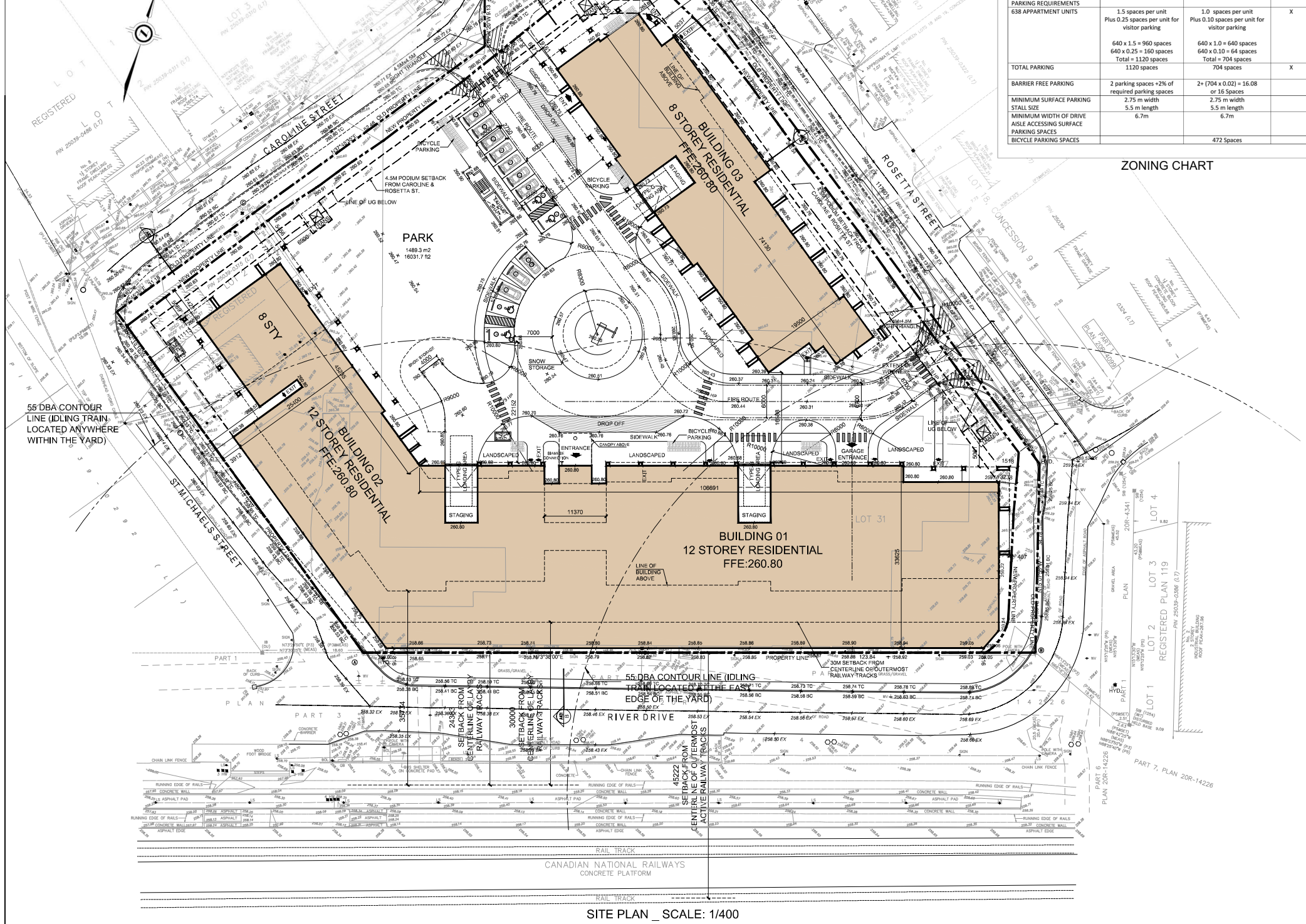
CLIENT 1 ROSETTA STREET (HALTON HILLS) GP LTD.		PROJECT NAME 1 ROSETTA STREET		 IBI GROUP Suite 200 - 360 James Street North Hamilton ON L8L 1H5 Canada tel 905 546 1010 fax 905 546 1011 ibigroup.com	
SCALE: NTS	DATE: 2022-02-25	FIGURE NAME AERIAL PLAN		FIGURE NO. FIG.1	REVISION 1
PROJECT ENG: JJ	DRAWN BY: CMM				
CHECKED BY: JJ	APPROVED BY: JJ				
PROJECT NO: 125082					

Appendix A – Background Information

Sample Architectural Drawings (Icon Architects)
Plan and Profile Drawings (Region of Halton)
Topographic Survey (J.D Barnes)



KEY PLAN _ NTS



SITE PLAN _ SCALE: 1/400

	DEVELOPMENT DETAILS HIGH DENSITY RESIDENTIAL ZONE (HDR) - EXCEPTION		
	Required	Proposed	Amendment Needed
MINIMUM LOT FRONTAGE (Rosetta Street)	11.0 m	117.60	
MIN. FRONT YARD (Rosetta Street)	4.5 m	0.93 m	X
MIN. REAR YARD (St. Michael's Street)	7.5 m	3.91 m	X
MIN. INTERIOR SIDE YARD (River Street)	7.5 m	0.81 m	X
MIN. SETBACK FROM A RAILWAY	30.0 m	30 m	X
MAX. BUILDING HEIGHT	25.0 m	Measured from the building face to the centerline of the active layby lane: Building 1 = 39.79M Building 2 = 39.79M Building 3 = 27.63M	X
MAX. LOT COVERAGE		52%	
OUTDOOR AMENITY AREA INCLUDING PRIVATE TERRACES AND BALCONIES		7006.30 sq.m	
MIN. OUTDOOR AMENITY AREA (PARK)		1,489.30 sq.m	
MIN. LANDSCAPE OPENSOURCE (%)		32.70 %	
MIN. PARKING SETBACK BETWEEN A BUILDING AND AN AT-GRADE PARKING AREA	1.2 M	11.78 M	
PARKING REQUIREMENTS			X
638 APARTMENT UNITS	1.5 spaces per unit	1.0 spaces per unit	
Plus 0.25 spaces per unit for visitor parking	Plus 0.25 spaces per unit for visitor parking	Plus 0.10 spaces per unit for visitor parking	
640 x 1.5 = 960 spaces	640 x 1.5 = 960 spaces	640 x 0.10 = 64 spaces	
640 x 0.25 = 160 spaces	640 x 0.25 = 160 spaces	Total = 704 spaces	
Total = 1120 spaces			
TOTAL PARKING		704 spaces	X
BARRIER FREE PARKING	2 parking spaces + 2% of required parking spaces	2+ (704 x 0.02) = 16.08	
MINIMUM SURFACE PARKING	2.75 m width	2.75 m width	
STALL SIZE	5.5 m length	5.5 m length	
MINIMUM WIDTH OF DRIVE	6.7m	6.7m	
ANGLE ACCESSING SURFACE PARKING SPACES			
BICYCLE PARKING SPACES		472 Spaces	

ZONING CHART

PROJECT STATISTICS

1 ROSETTA STREET

PRELIMINARY STATS	m2	ft2	hectare
Gross Site Area	14355.50	154,521.2	1.44
Net Site Area	13,248.80	144,503.2	1.34
Total GFA	48,276.00	518,638.0	
FSI (based on Gross Site Area)	3.36		
Density (units/hectare)	476.73		
Total Units	640		
Lot Coverage	52%		
Landscape Area	32.7%	4,388.60	47,238.5

Floors	GFA		Units						Total
	m2	ft2	1B	1B+D+2BTH	1B+D+2BTH	2B+2BTH	2B+D+2BTH	3B	
UG2	484.00	5,209.7							
UG1	516.20	5,556.3							
GROUND FLOOR	1,151.20	12,391.4							
2ND FLOOR	1,464.70	15,765.9	5	6	4	2	2	1	20
3RD FLOOR	1,608.20	17,310.5	5	8	4	3	2	1	23
4TH FLOOR	1,608.20	17,310.5	5	8	4	3	2	1	23
5TH FLOOR	1,608.20	17,310.5	5	8	4	3	2	1	23
6TH FLOOR	1,608.20	17,310.5	5	8	4	3	2	1	23
7TH FLOOR	1,608.20	17,310.5	5	8	4	3	2	1	23
8TH FLOOR	1,614.10	17,374.0	4	9	4	3	2	1	23
9TH FLOOR	1,614.10	17,374.0	4	9	4	3	2	1	23
10TH FLOOR	1,614.10	17,374.0	4	9	4	3	2	1	23
11TH FLOOR	1,614.10	17,374.0	4	9	4	3	2	1	23
12TH FLOOR	1,614.10	17,374.0	4	9	4	3	2	1	23
Total	19,727.60	212,345.9	50	91	44	32	22	11	230

Floors	GFA		Units						Total
	m2	ft2	1B	1B+D+2BTH	1B+D+2BTH	2B+2BTH	2B+D+2BTH	3B	
UG2	331.70	3,570.4							
UG1	372.70	4,011.7							
GROUND FLOOR	1,002.30	10,788.7							
2ND FLOOR	1,491.90	16,068.7	6	7	4	1	2	2	22
3RD FLOOR	1,540.00	16,576.4	6	8	4	1	2	2	23
4TH FLOOR	1,540.00	16,576.4	6	8	4	1	2	2	23
5TH FLOOR	1,540.00	16,576.4	6	8	4	1	2	2	23
6TH FLOOR	1,540.00	16,576.4	6	8	4	1	2	2	23
7TH FLOOR	1,540.00	16,576.4	6	8	4	1	2	2	23
8TH FLOOR	1,488.30	16,019.9	7	9	4	1	2	0	23
9TH FLOOR	1,438.80	15,487.1	4	9	4	1	2	0	21
10TH FLOOR	1,333.50	14,353.7	3	7	4	2	3	0	19
11TH FLOOR	1,333.50	14,353.7	3	7	4	2	3	0	19
12TH FLOOR	1,278.90	13,766.0	4	8	4	1	2	0	19
Total	17,717.60	191,291.7	57	87	44	14	24	12	238

Floors	GFA		Units						Total
	m2	ft2	1B	1B+D+2BTH	1B+D+2BTH	2B+2BTH	2B+D+2BTH	3B	
UG2	374.90	4,022.6							
UG1	466.90	5,023.7							
GROUND FLOOR	1,336.30	14,383.8	4	3	2	0	1	1	11
2ND FLOOR	1,248.60	13,439.8	5	8	2	2	1	1	20
3RD FLOOR	1,304.40	14,040.4	5	8	3	2	2	1	21
4TH FLOOR	1,304.40	14,040.4	5	8	3	2	2	1	21
5TH FLOOR	1,304.40	14,040.4	5	8	3	2	2	1	21
6TH FLOOR	1,304.40	14,040.4	5	8	3	2	2	1	21
7TH FLOOR	1,304.40	14,040.4	5	8	3	2	2	1	21
8TH FLOOR	1,028.10	11,066.4	3	7	3	2	1	0	16
Total	10,776.80	116,000.4	37	58	22	14	14	7	152

Floors	BUILDING 1		BUILDING 2		BUILDING 3		TOTAL GFA	
	m2	ft2	m2	ft2	m2	ft2	m2	ft2
UG2	484.00	5,209.7	331.70	3,570.4	374.90	4,022.6	1,190.60	12,802.7
UG1	516.20	5,556.3	372.70	4,011.7	466.90	5,023.7	1,355.80	14,593.7
GROUND FLOOR	1,151.20	12,391.4	1,002.30	10,788.7	1,336.30	14,383.8	3,489.80	37,563.9
2ND FLOOR	1,464.70	15,765.9	1,491.90	16,068.7	1,248.60	13,439.8	4,205.20	45,264.4
3RD FLOOR	1,608.20	17,310.5	1,540.00	16,576.4	1,304.40	14,040.4	4,452.60	47,927.3
4TH FLOOR	1,608.20	17,310.5	1,540.00	16,576.4	1,304.40	14,040.4	4,452.60	47,927.3
5TH FLOOR	1,608.20	17,310.5	1,540.00	16,576.4	1,304.40	14,040.4	4,452.60	47,927.3
6TH FLOOR	1,608.20	17,310.5	1,540.00	16,576.4	1,304.40	14,040.4	4,452.60	47,927.3
7TH FLOOR	1,608.20	17,310.5	1,540.00	16,576.4	1,304.40	14,040.4	4,452.60	47,927.3
8TH FLOOR	1,614.10	17,374.0	1,488.30	16,019.9	1,028.10	11,066.4	4,130.50	44,460.3
9TH FLOOR	1,614.10	17,374.0	1,438.80	15,487.1			3,052.90	32,861.1
10TH FLOOR	1,614.10	17,374.0	1,333.50	14,353.7			2,947.60	31,727.7
11TH FLOOR	1,614.10	17,374.0	1,333.50	14,353.7			2,947.60	31,727.7
12TH FLOOR	1,614.10	17,374.0	1,278.90	13,766.0			2,893.00	31,140.0
Total GFA	19,727.60	212,345.9	17,717.60	191,291.7	10,776.80	116,000.4	48,276.00	518,638.0

PARKING REQUIRED	Units		Ratio	Required
	Residential	Visitor		
Residential	640	0.1	1.0	640
Visitor	640	0.1		64
TOTAL				704

PARKING PROVIDED	Visitors		Residents	
	Grade	UG	UG1	UG2
Grade	64			22
UG1		0	329	
UG2		0	292	
TOTAL		64	621	

ACCESSIBLE PARKING AS PER AODA	RATIO	REQUIRED PARKING	REQUIRED ACCESSIBLE PARKING	PROVIDED ACCESSIBLE PARKING
2 PARKING SPACES + 2% OF REQUIRED PARKING SPACES		704	16	16
TOTAL				16

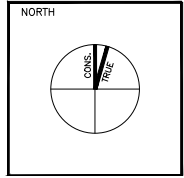
FLOOR AREA, GROSS
The aggregate of the areas of each floor of a building or structure above or below established grade, measured between the exterior faces of the exterior walls of the building or structure excluding the sum of the areas of each floor used, or designed or intended for use for the parking of motor vehicles, unless the parking of motor vehicles is the principle use of the building or structure.

FLOOR AREA, NET
The aggregate of the floor areas of a building above or below established grade, but excluding car parking areas within the building, stairways, elevator shafts, service or mechanical rooms and penthouses, washrooms, garbage or recycling rooms, staff locker and lunch rooms, loading areas, any space with a floor to ceiling height of less than 1.8 metres and any part of a basement that is unfinished, is used solely for storage purposes and is not accessible to the public.

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NO.	DATE	DESCRIPTION



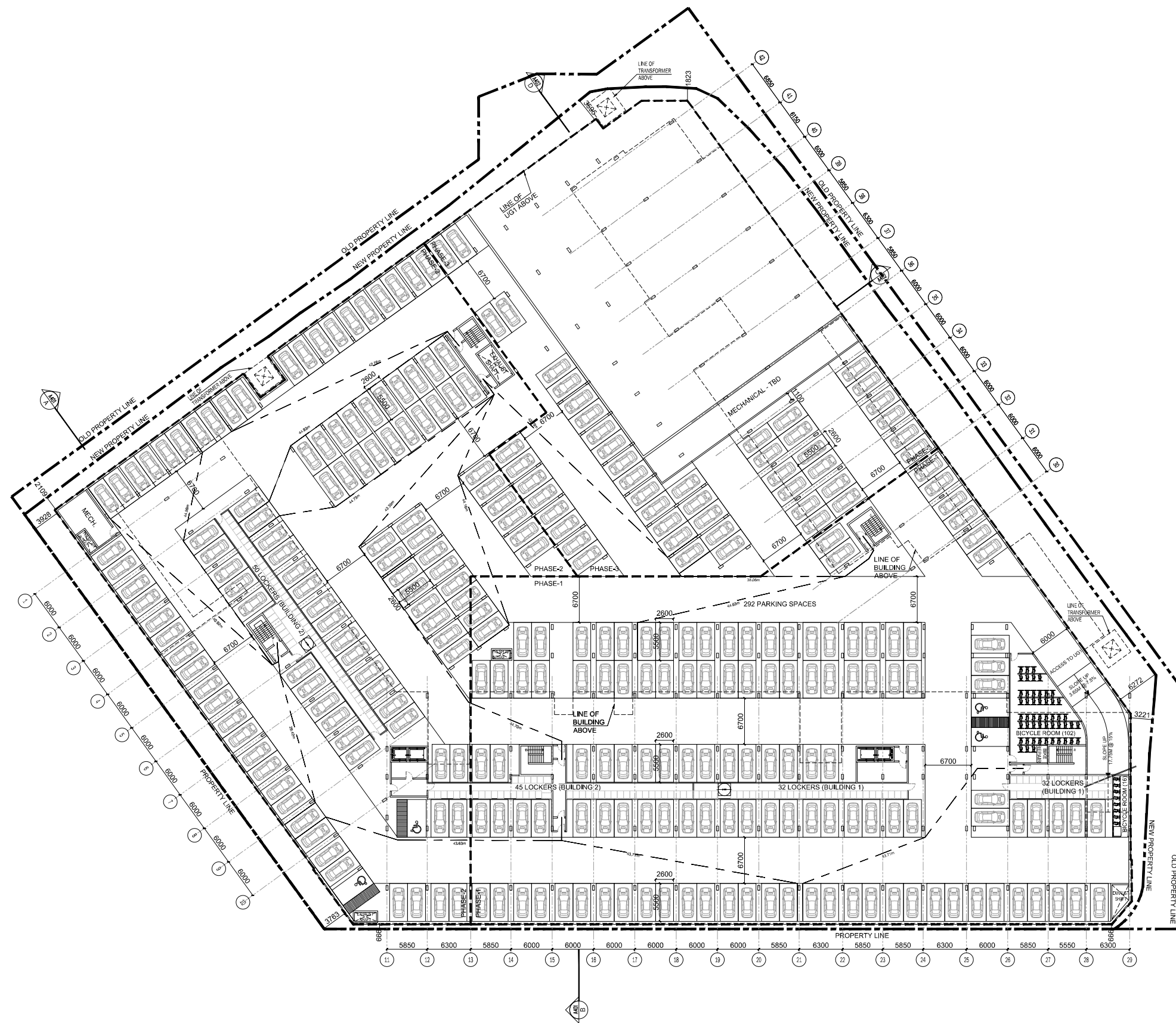
1 ROSETTA STREET
GEORGETOWN, ON

DRAWING TITLE
SITE PLAN & PROJECT STATS

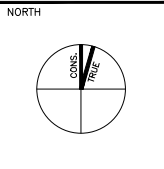
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Date: MAR.08, 2022
Project No. 17127

Drawn by: RK
Checked by: RE
Drawing No. A101

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GEORGETOWN, ON**

DRAWING TITLE
UG2 FLOOR PLAN

Scale:
1/300

Date:
MAR.09, 2022

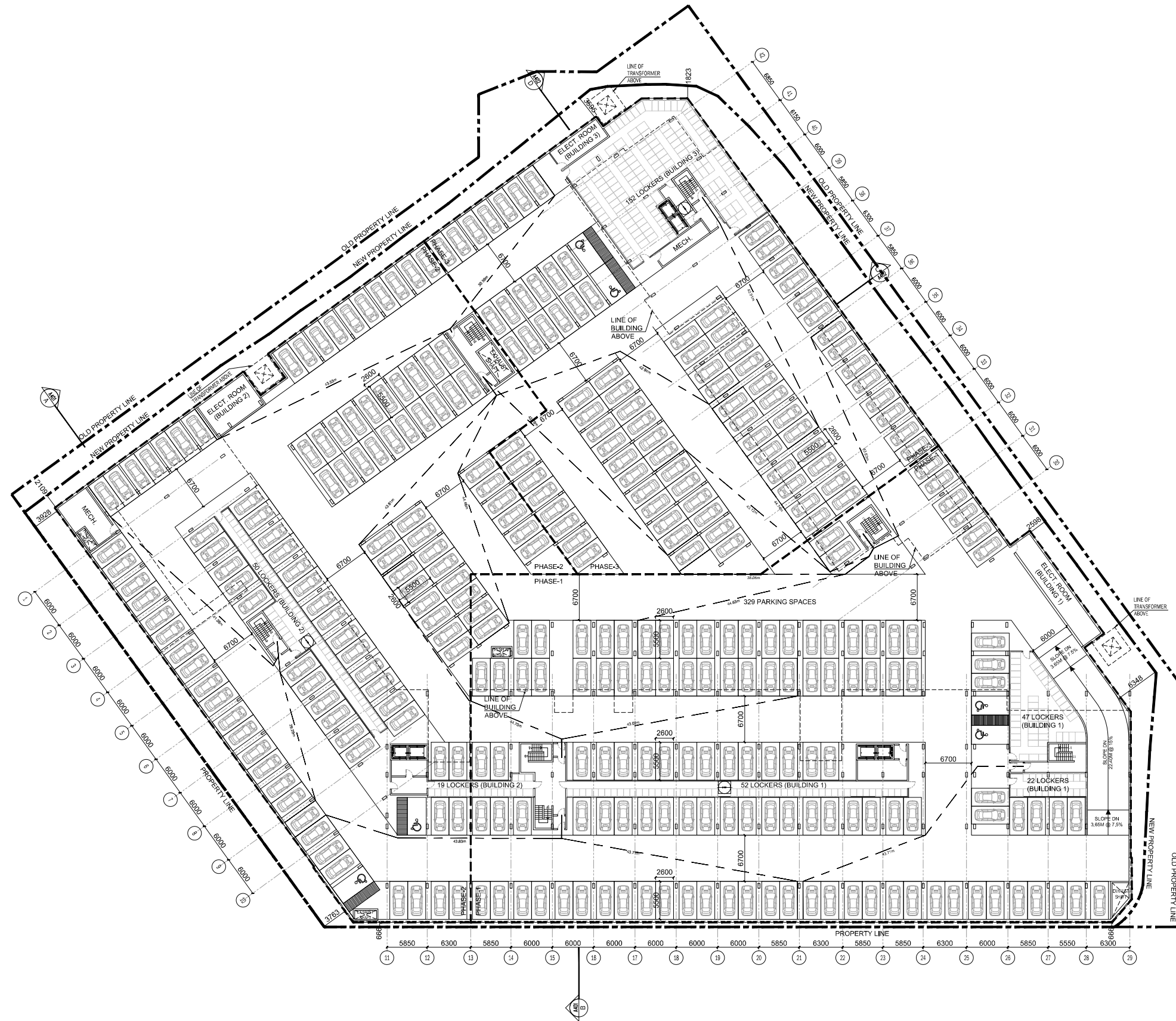
Project No.
17127

Drawn by:
RK

Checked by:
RE

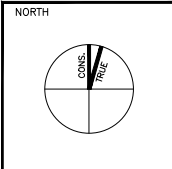
Drawing No.
A201

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**1 ROSETTA STREET
GEORGETOWN, ON**

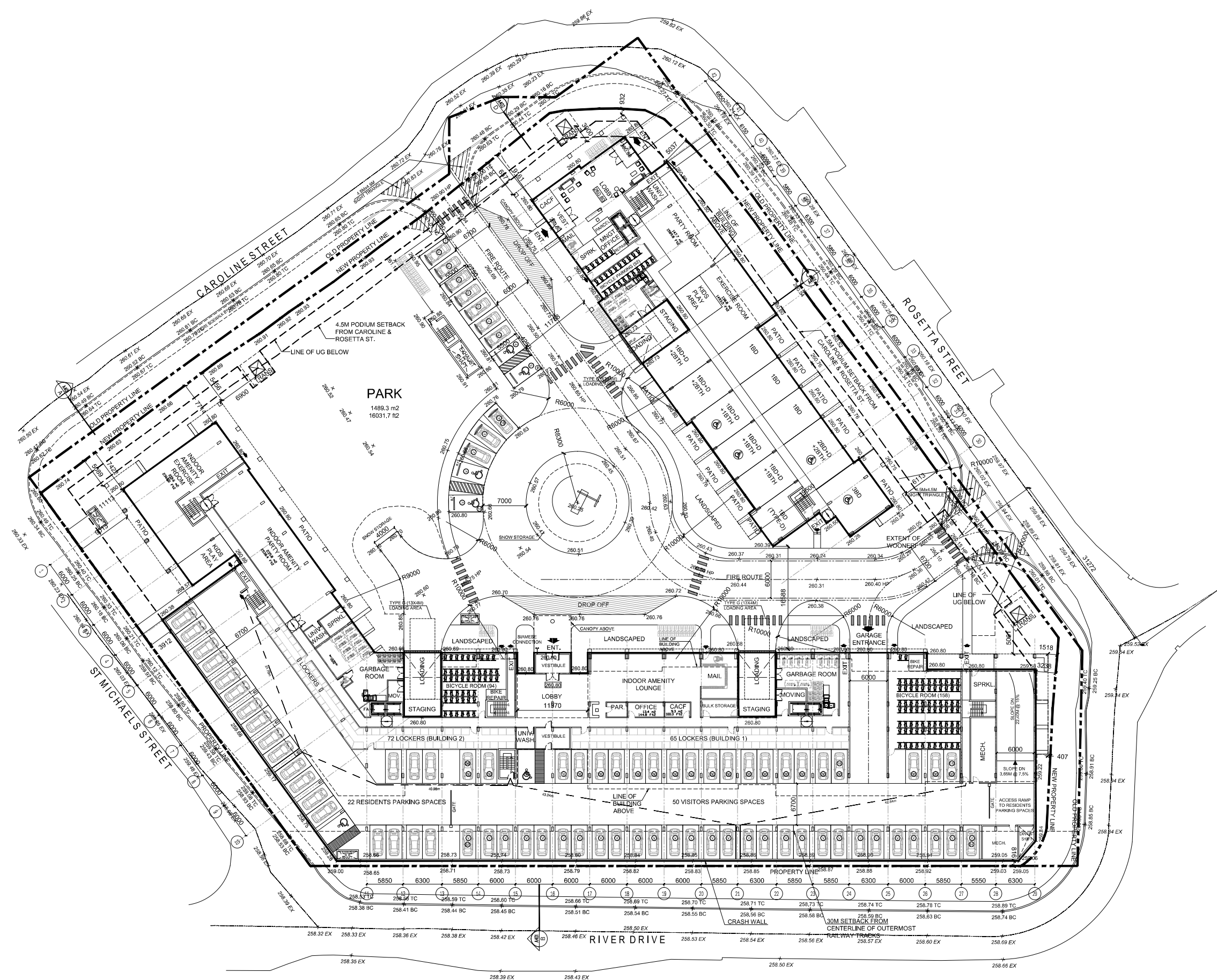
UG1 FLOOR PLAN

Scale: 1/300
 Date: MAR.08, 2022
 Project No. 17127

Drawn by: RK
 Checked by: RE
 Drawing No. A202

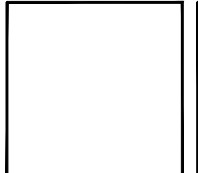
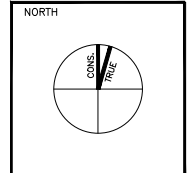
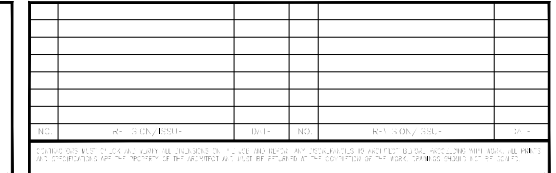
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NO.	DATE	DESCRIPTION	BY	CHECKED



**1 ROSETTA STREET
GEORGETOWN, ON**

DRAWING TITLE
GROUND FLOOR PLAN

Scale:
1/300

Date:
MAR.08, 2022

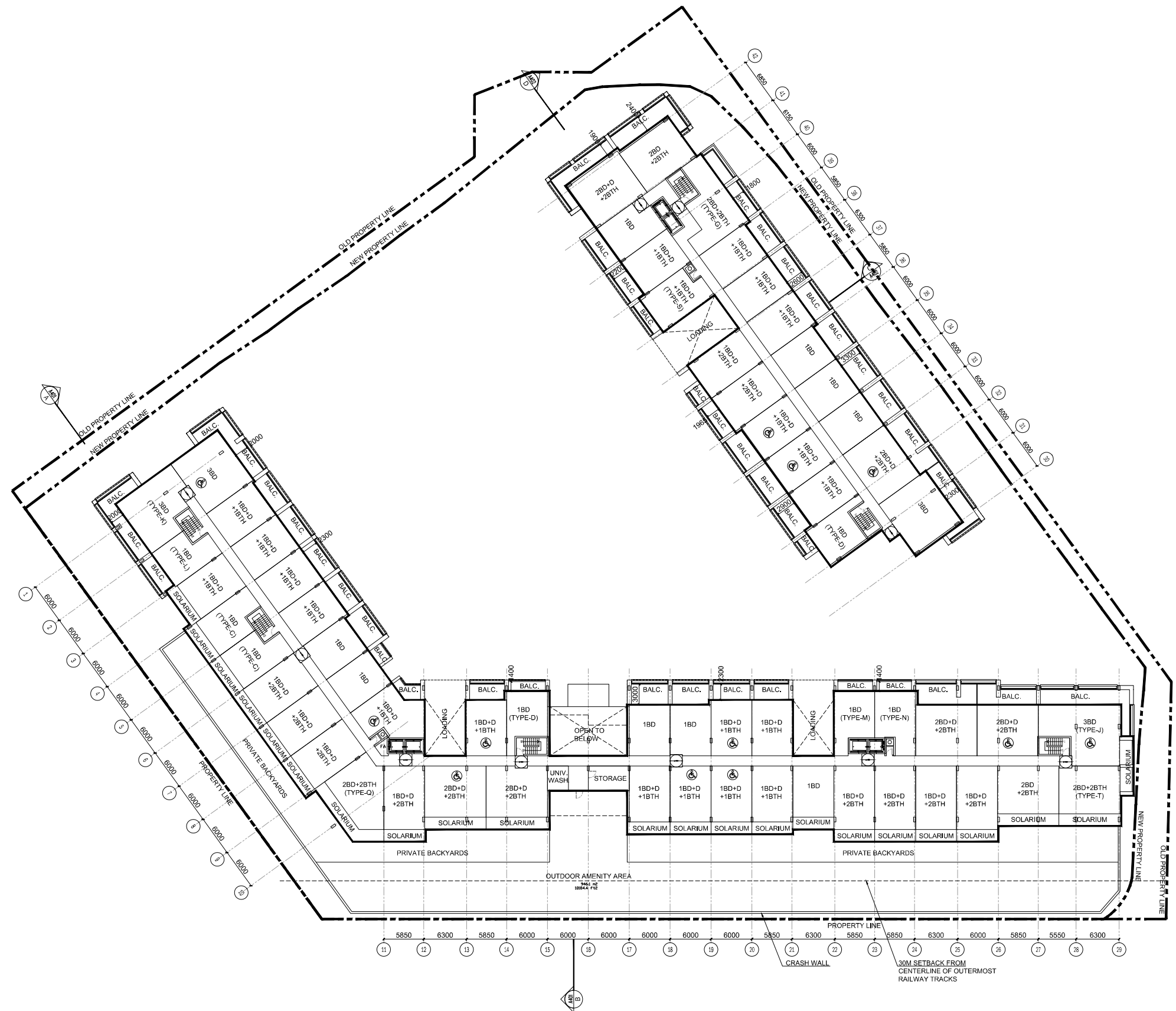
Project No.
17127

Drawn by:
RK

Checked by:
RE

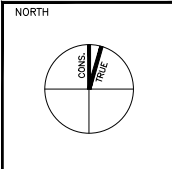
Drawing No.
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NO.	DATE	DESCRIPTION



1 ROSETTA STREET
GEORGETOWN, ON

DRAWING TITLE
2ND FLOOR PLAN

Scale:
1/300

Date:
MAR.08, 2022

Project No.
17127

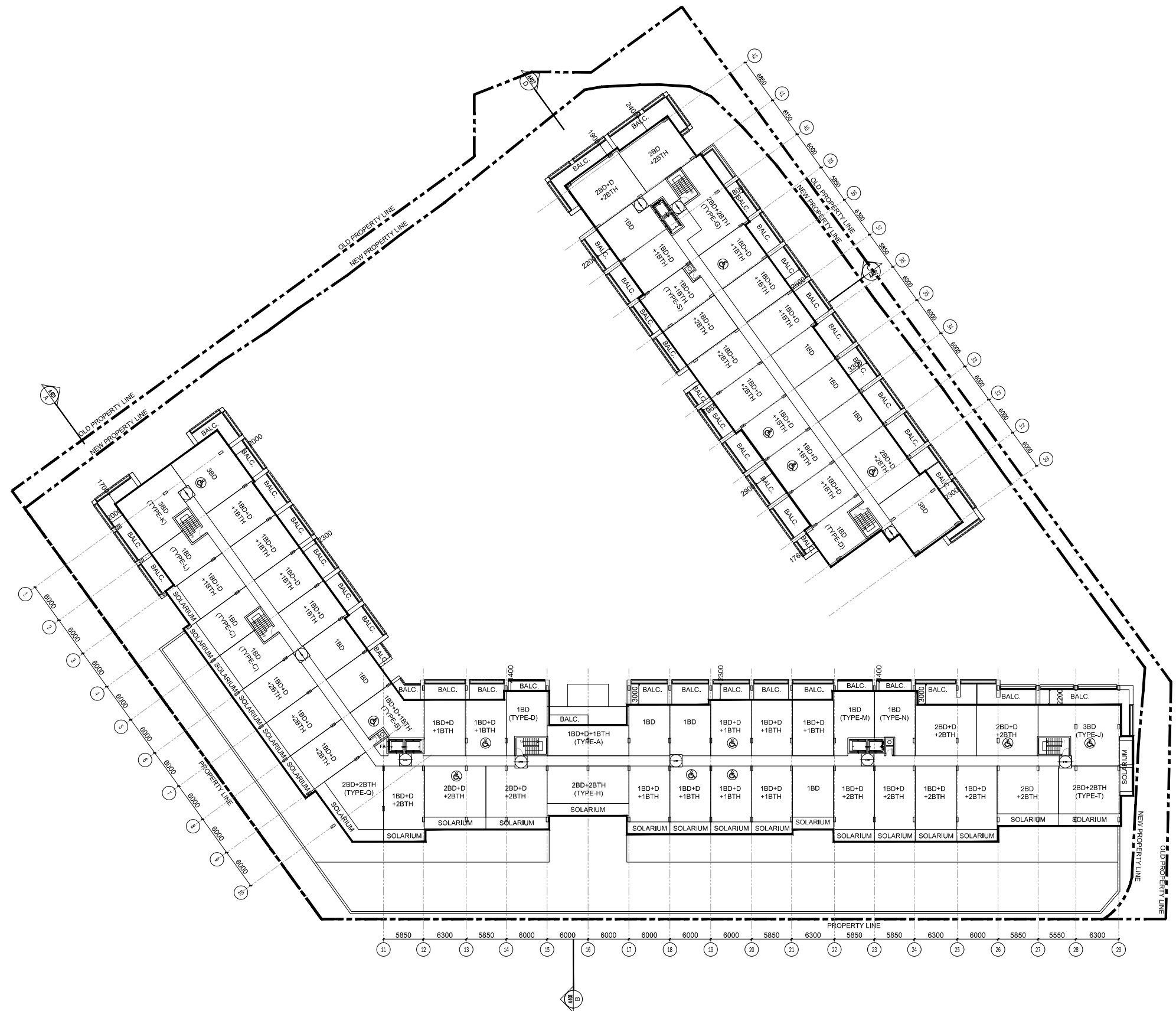
Drawn by:
RK

Checked by:
RE

Drawn No.
A204

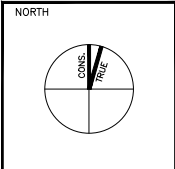
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NO.	DATE	DESCRIPTION	BY	CHKD.



**1 ROSETTA STREET
GEORGETOWN, ON**

DRAWING TITLE
3RD FLOOR PLAN

Scale:
1/300

Date:
MAR.09, 2022

Project No.
17127

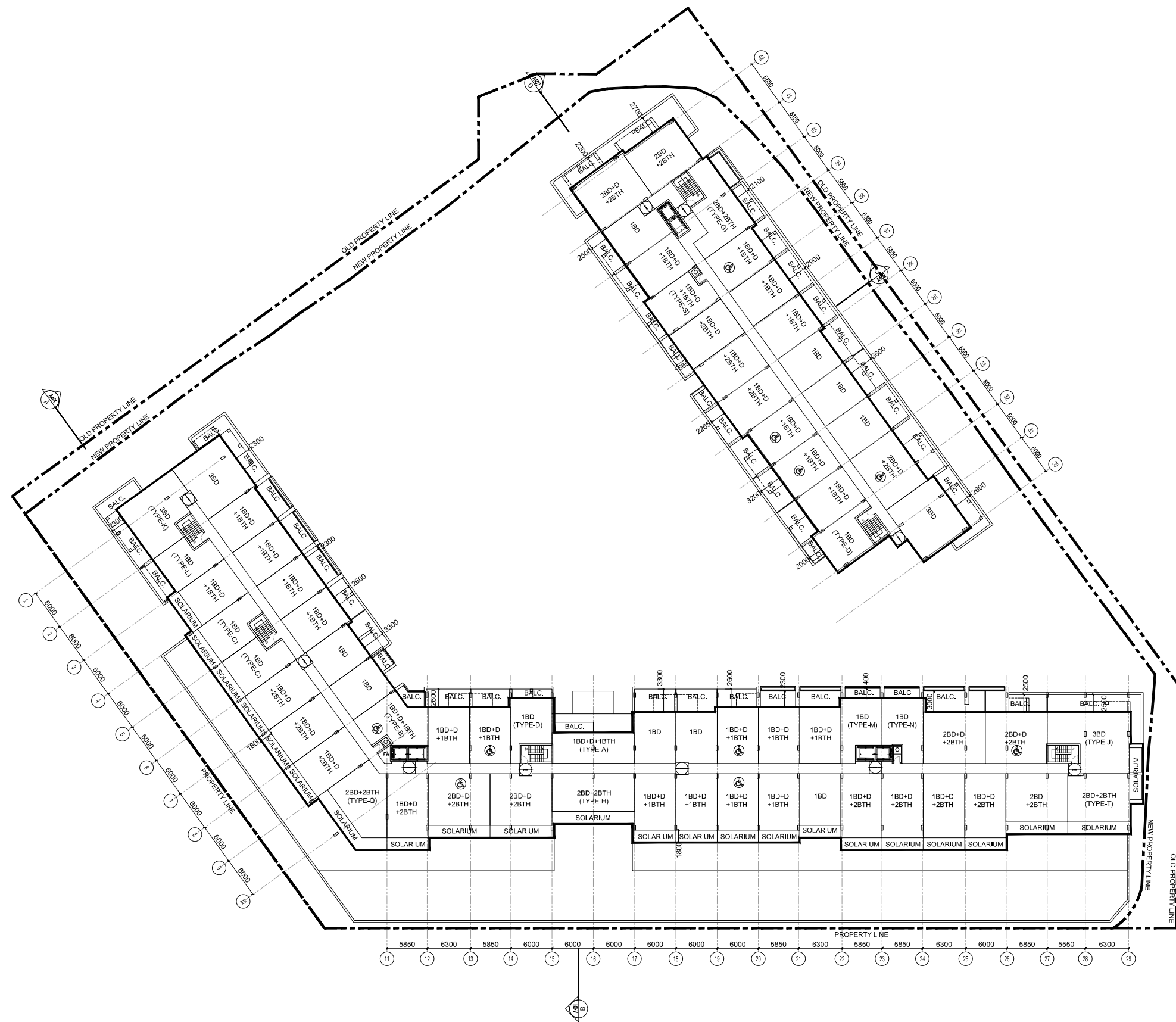
Drawn by:
RK

Checked by:
RE

Drawing No.
A205

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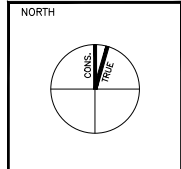
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NO.	DATE	DESCRIPTION	BY	CHECKED

NO.	DATE	DESCRIPTION	BY	CHECKED



1 ROSETTA STREET
GEORGETOWN, ON

DRAWING TITLE
4TH FLOOR PLAN

Scale:
1/300

Date:
MAR.05, 2022

Project No.
17127

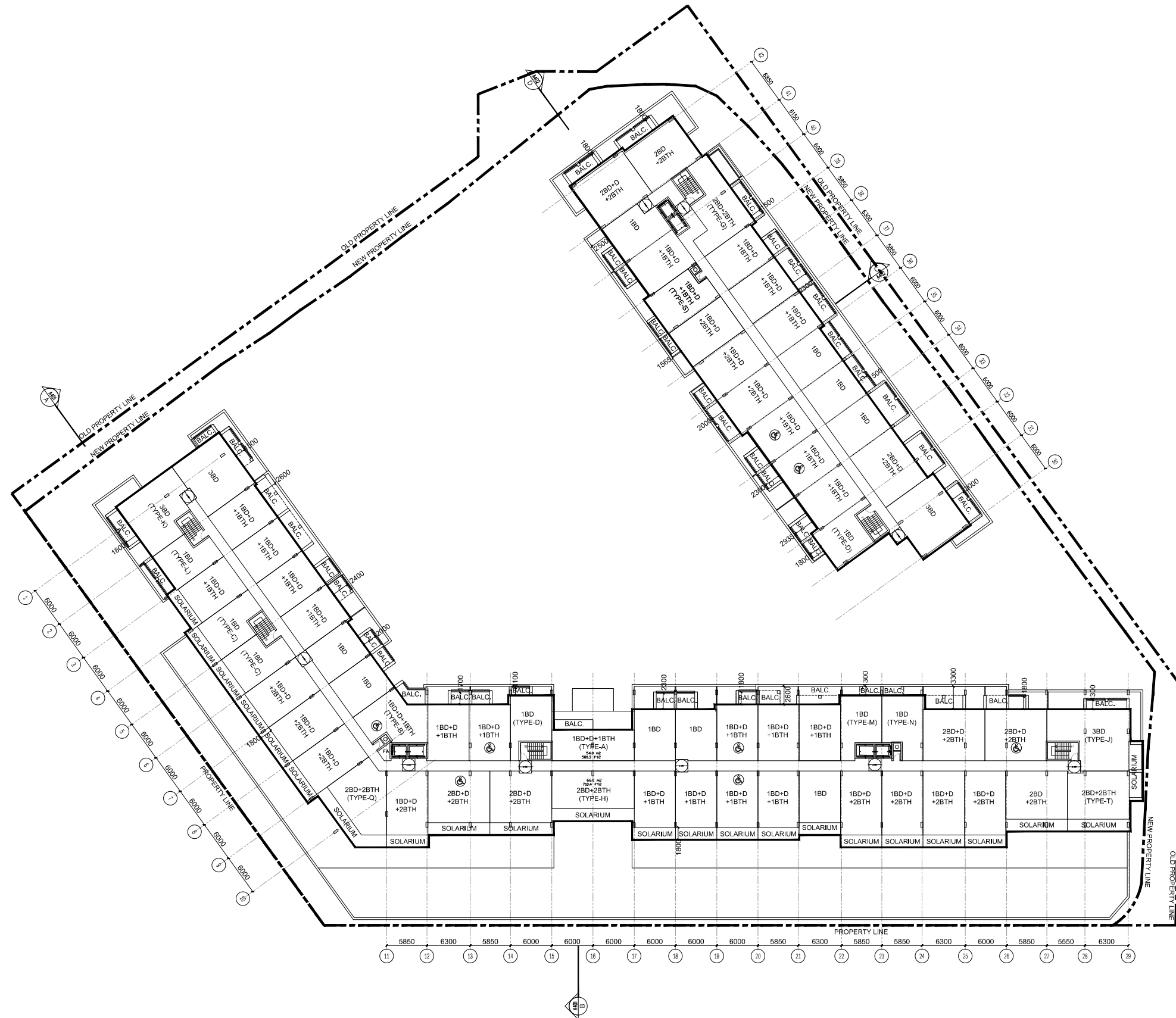
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RK

Checked by:
RE

Drawn No.
A206

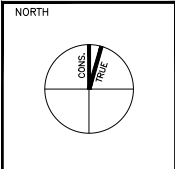
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**1 ROSETTA STREET
GEORGETOWN, ON**

DRAWING TITLE
5TH FLOOR PLAN

Scale:
1/300

Date:
MAR.09, 2022

Project No.
17127

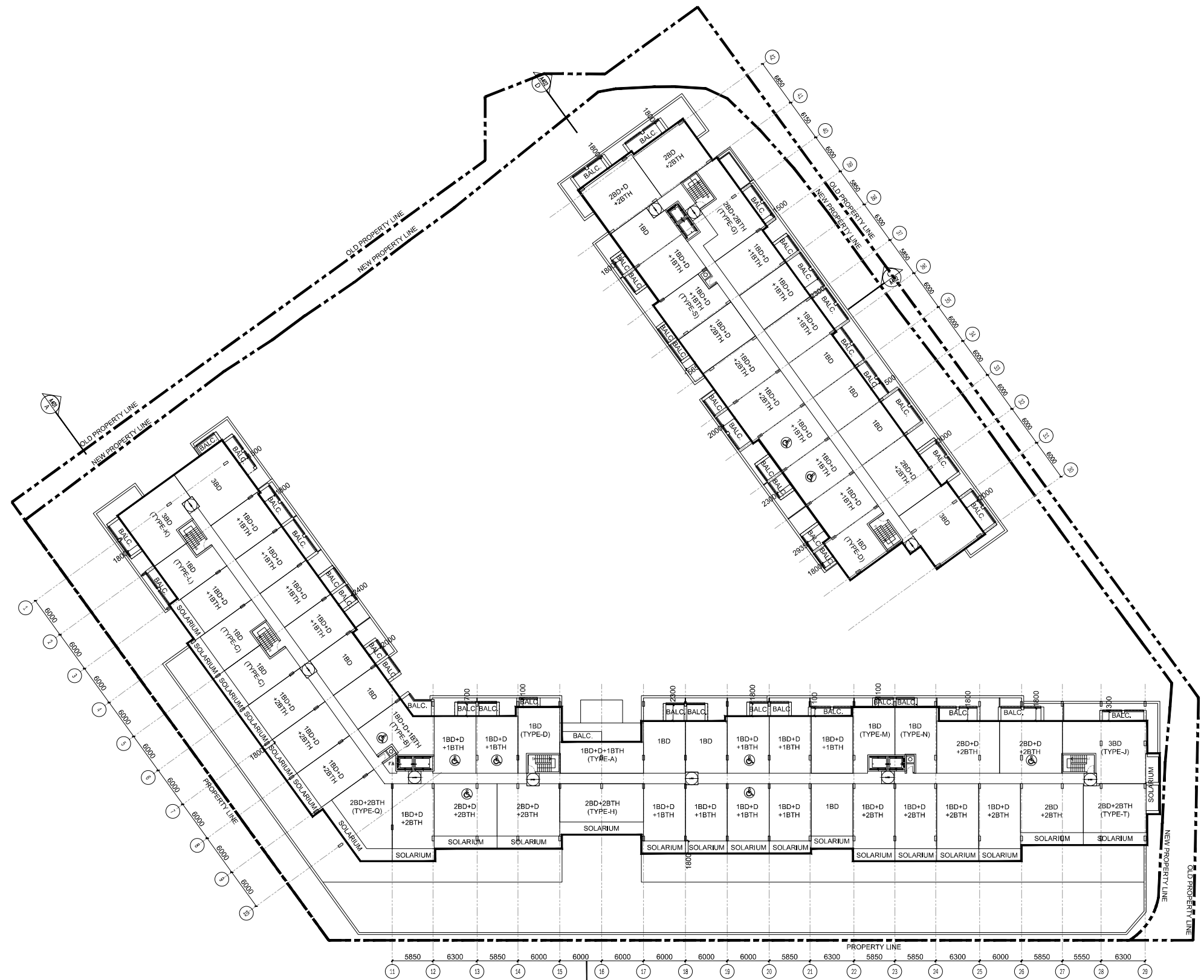
Drawn by:
RK

Checked by:
RE

Drawing No.
A207

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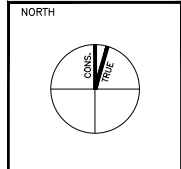




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NO.	DATE	BY	DESCRIPTION

NO.	DATE	BY	DESCRIPTION



**1 ROSETTA STREET
GEORGETOWN, ON**

DRAWING TITLE
6TH & 7TH FLOOR PLANS

Scale:
1/300

Date:
MAR.08, 2022

Project No.
17127

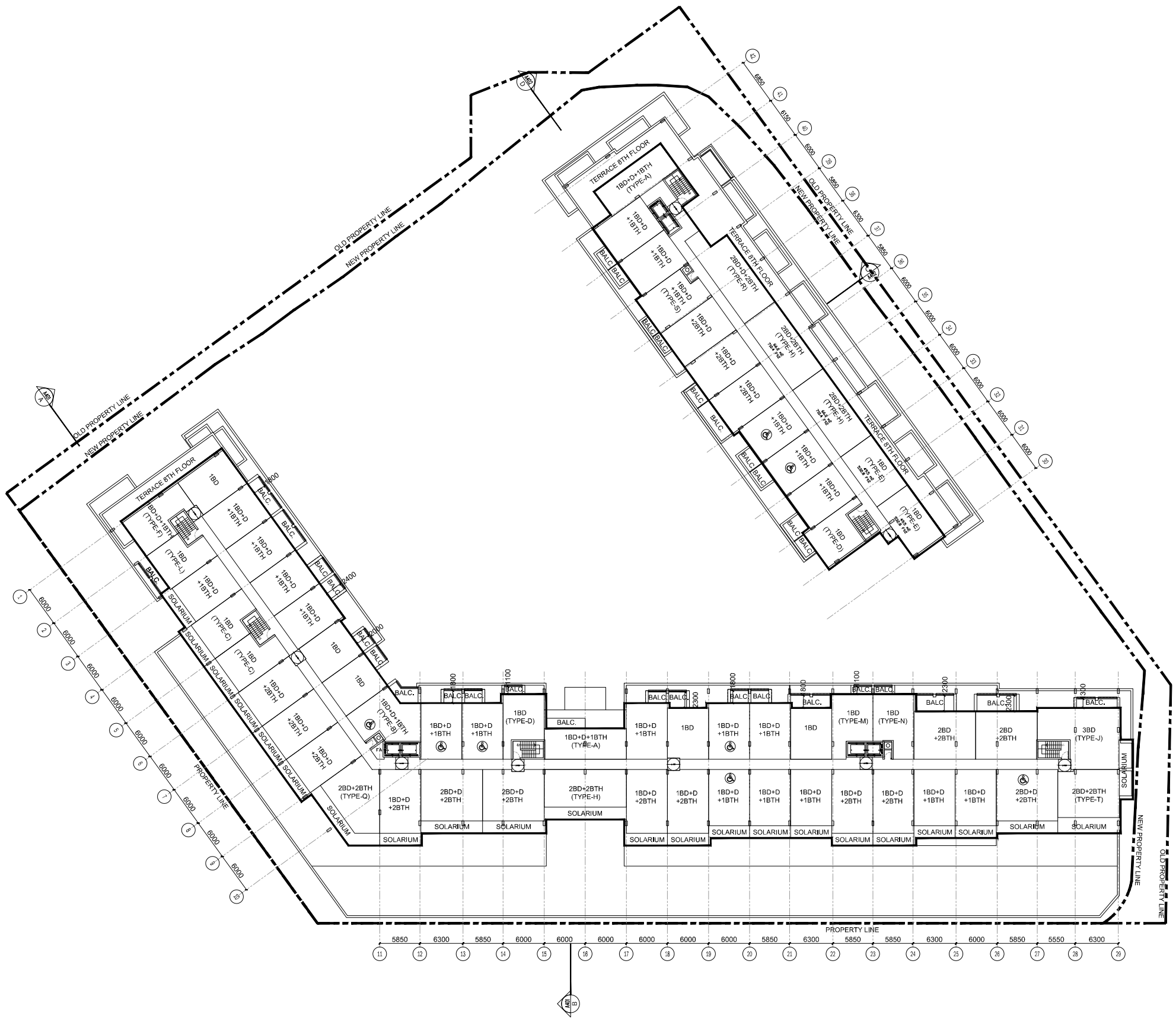
Drawn by:
RK

Checked by:
RE

Drawing No.
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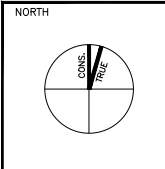
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NO.	DATE	BY	REVISION



1 ROSETTA STREET
GEORGETOWN, ON

DRAWING TITLE
8TH FLOOR PLAN

Scale:
1/300

Date:
MAR.08, 2022

Project No.
17127

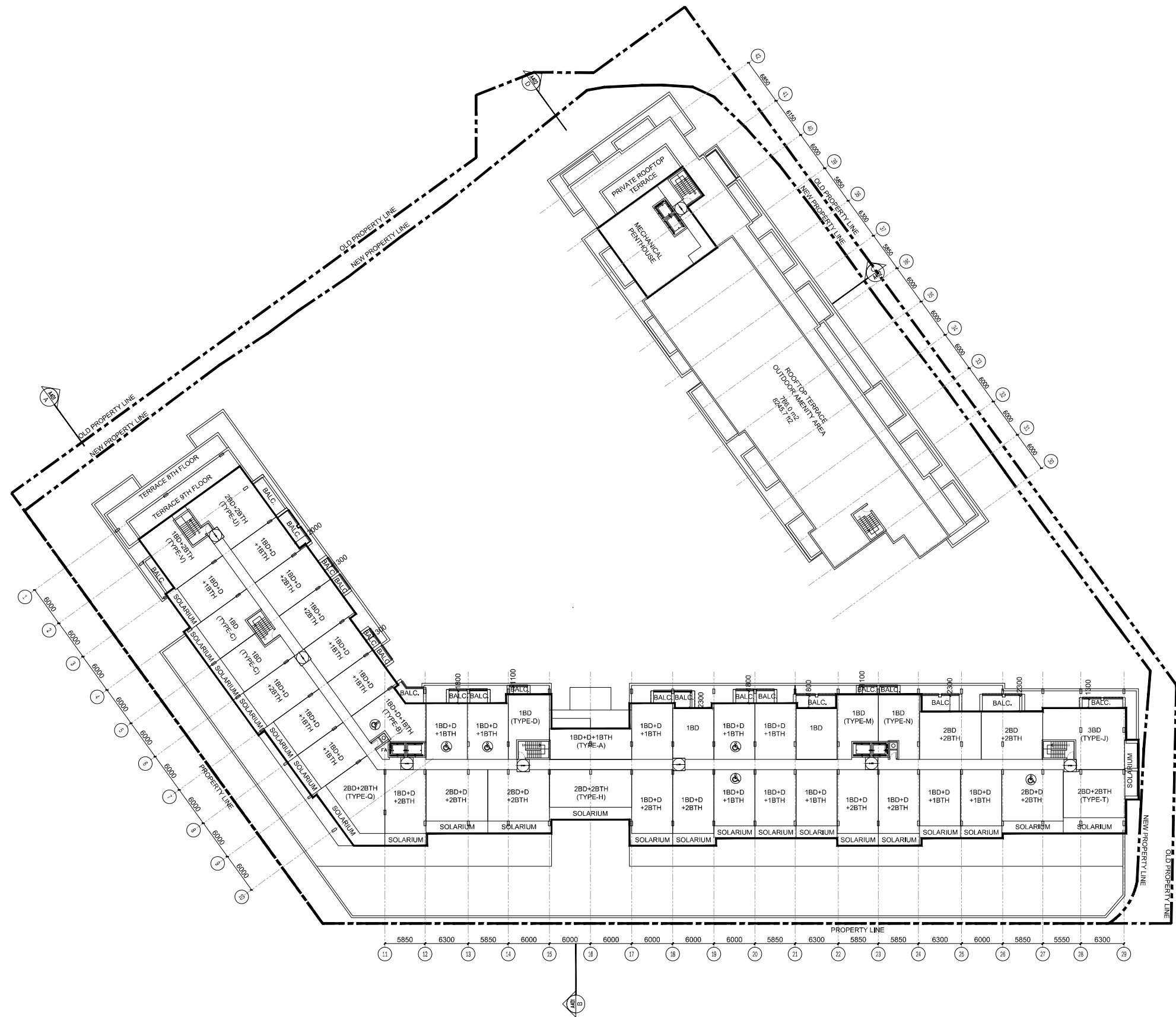
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Checked by:
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Drawing No.
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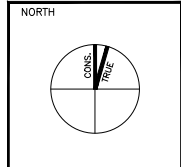
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NO.	DATE	BY	REVISION



1 ROSETTA STREET
GEORGETOWN, ON

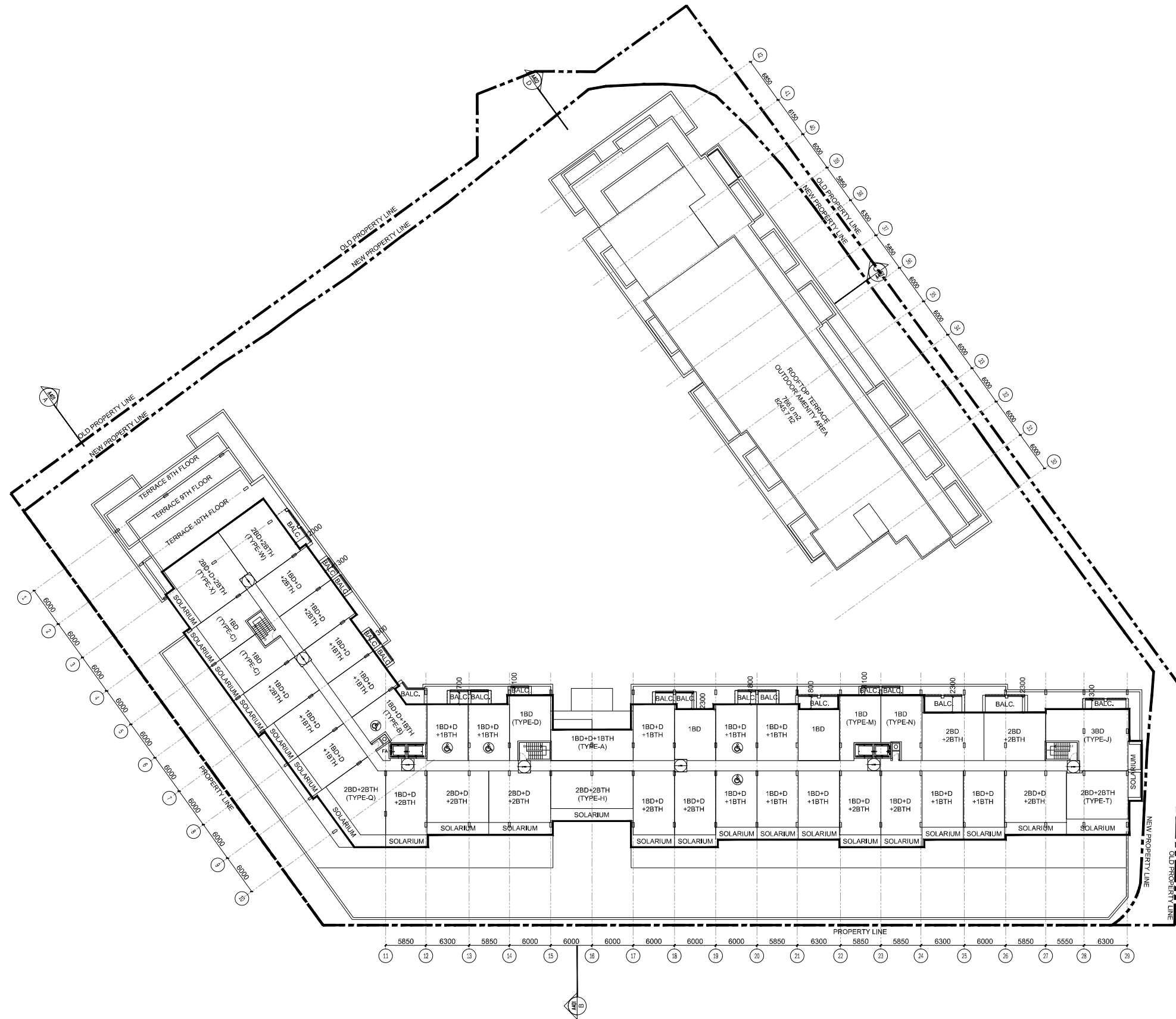
DRAWING TITLE
9TH FLOOR PLAN

Scale: 1/300
Date: MAR.09, 2022
Project No. 17127

Drawn by: RK
Checked by: RE
Drawing No. A210

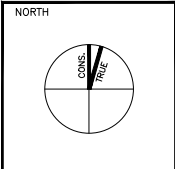
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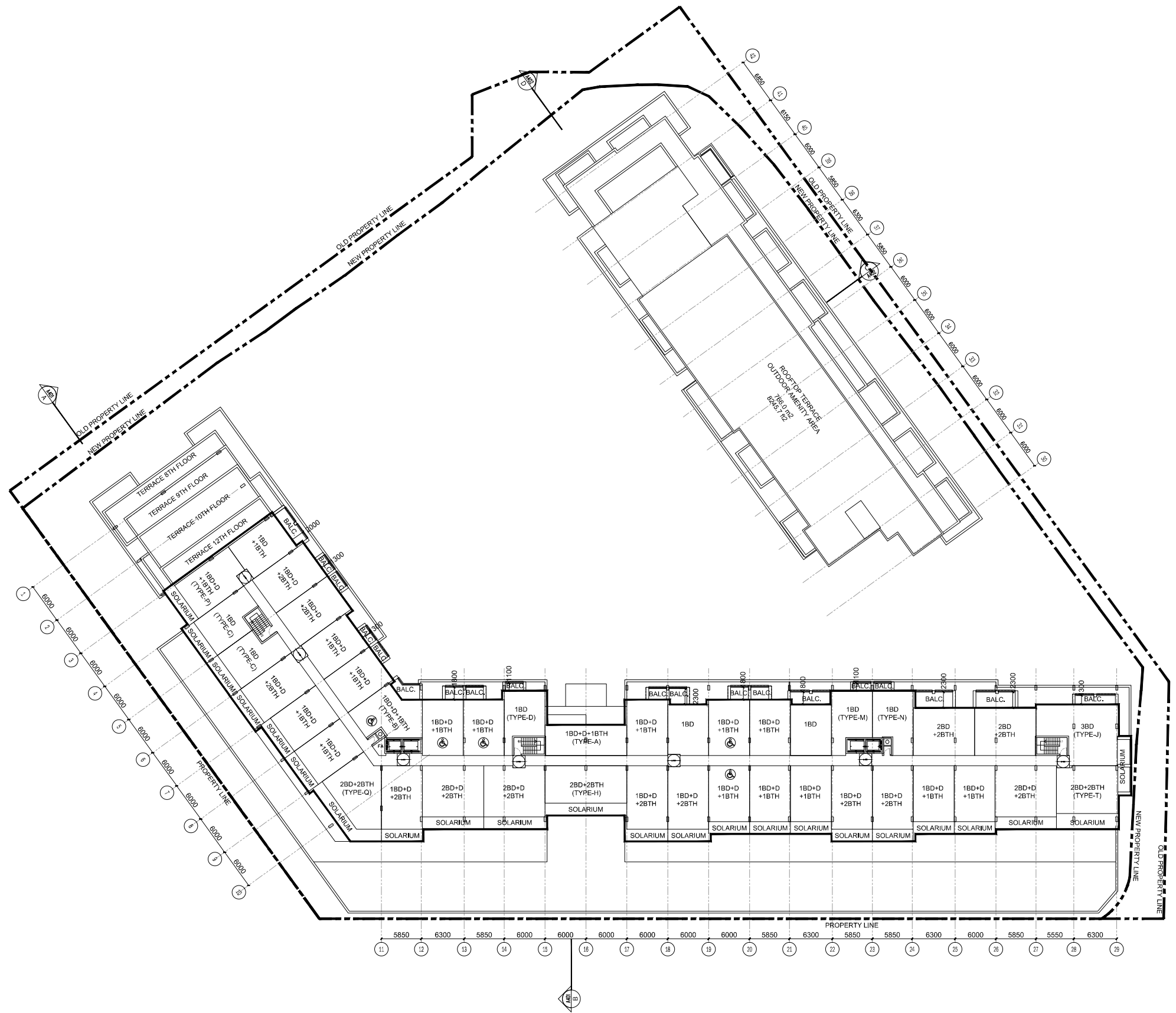
1 ROSETTA STREET
GEORGETOWN, ON

DRAWING TITLE
10TH-11TH TYPICAL FLOOR PLAN
Scale: 1/300
Date: MAR.08, 2022
Project No. 17127

Drawn by: RK
Checked by: RE
Drawing No. A211

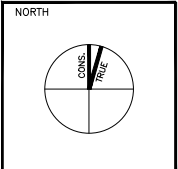


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1 ROSETTA STREET
GEORGETOWN, ON

DRAWING TITLE
12TH FLOOR PLAN

Scale:
1/300

Date:
MAR.08, 2022

Project No.
17127

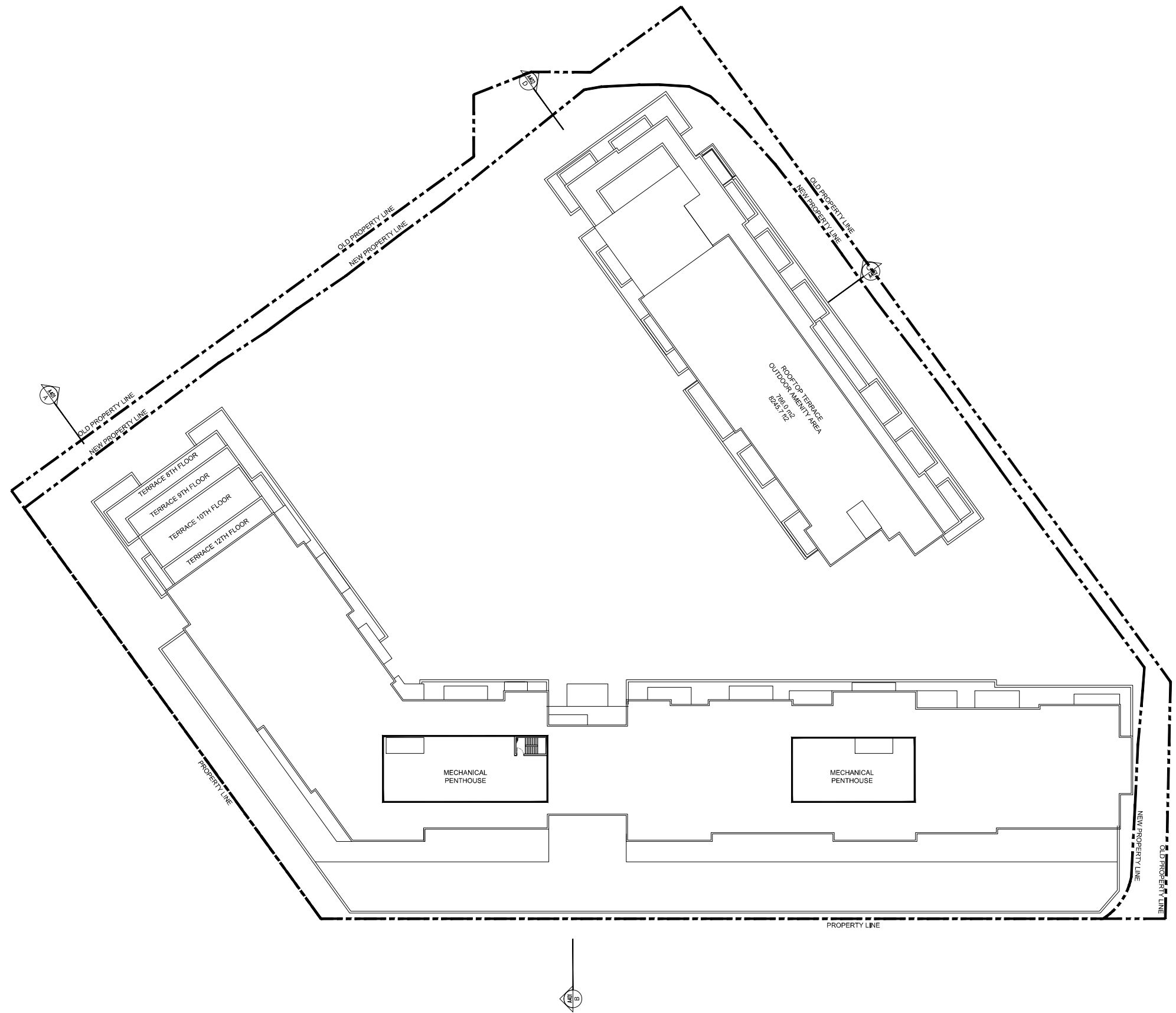
Drawn by:
RK

Checked by:
RE

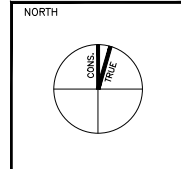
Drawing No.
A212

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1 ROSETTA STREET
GEORGETOWN, ON

1 ROSETTA STREET
GEORGETOWN, ON

DRAWING TITLE
ROOF FLOOR PLAN

Scale:
1/300

Date:
MAR.08, 2022

Project No.
17127

Drawn by:
RK

Checked by:
RE

Drawing No.
A213

813 - 4788 YONGE ST. TORONTO
M5N 0G3 ICNARCHITECTS.CA
T:416-524-6505 F:416-524-6504





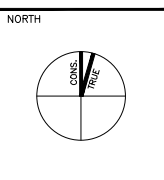
BUILDING - 1&2 _ NORTH ELEVATION _ SCALE 1:200



BUILDING - 1&2 _ SOUTH ELEVATION _ SCALE 1:200

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NO.	DATE	DESCRIPTION	BY	CHECKED



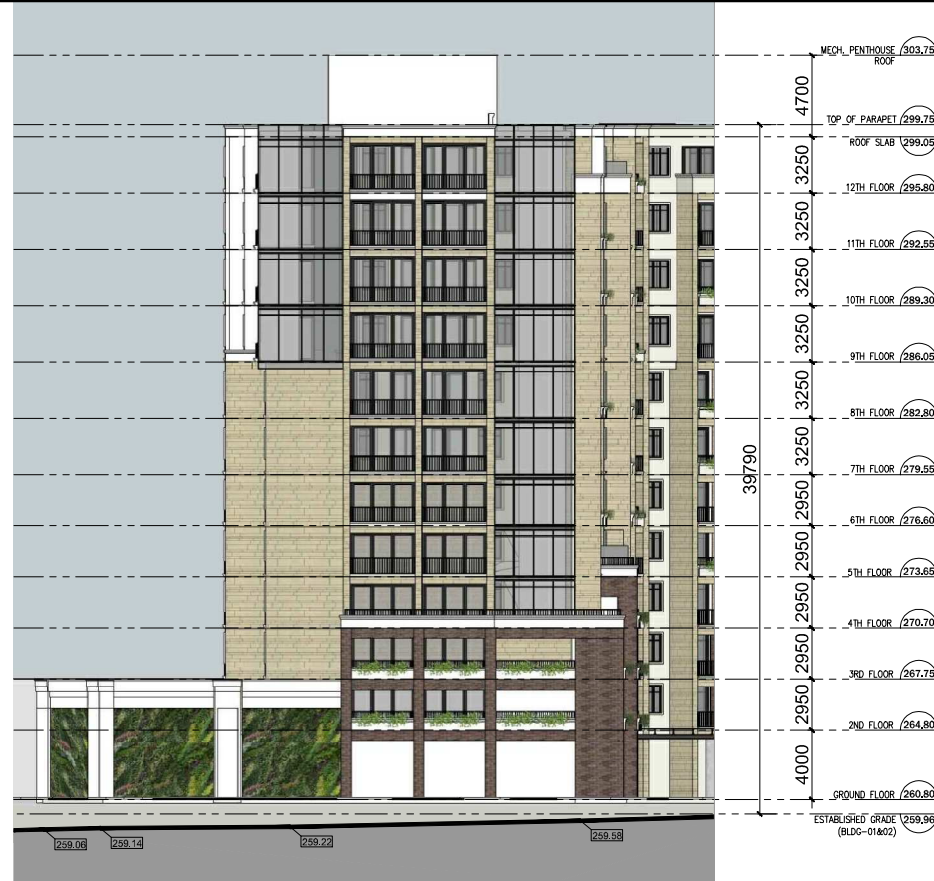
1 ROSETTA STREET
GEORGETOWN, ON

DRAWING TITLE
ELEVATIONS (BUILDINGS-1&2)
Scale:
1/200
Date:
MAR.08, 2022
Project No.
17127

Drawn by:
RK
Checked by:
RE
Drawing No.
A301

813 - 4788 YONGE ST. TORONTO
M5N 0G3 ICONARCHITECTS.CA
T:416-524-4505 F:416-524-4504





BUILDING - 1 _ EAST ELEVATION _ SCALE 1:200



BUILDING - 2 _ EAST ELEVATION _ SCALE 1:200



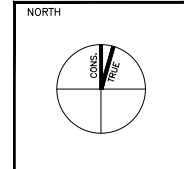
BUILDING - 2 _ NORTH ELEVATION _ SCALE 1:200



BUILDING - 2 _ WEST ELEVATION _ SCALE 1:200

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NO.	DATE	DESCRIPTION	BY	CHECKED



1 ROSETTA STREET
GEORGETOWN, ON

1 ROSETTA STREET
GEORGETOWN, ON

1 ROSETTA STREET
GEORGETOWN, ON

DRAWING TITLE
ELEVATIONS (BUILDINGS-1&2)
Scale: 1/200
Date: MAR.08, 2022
Project No. 17127
Drawn by: RK
Checked by: RE
Drawing No. A302

819 - 4789 YONGE ST. TORONTO
M5N 0G3 CANARCHITECTS.CA
T:416-524-6505 F:416-524-6504





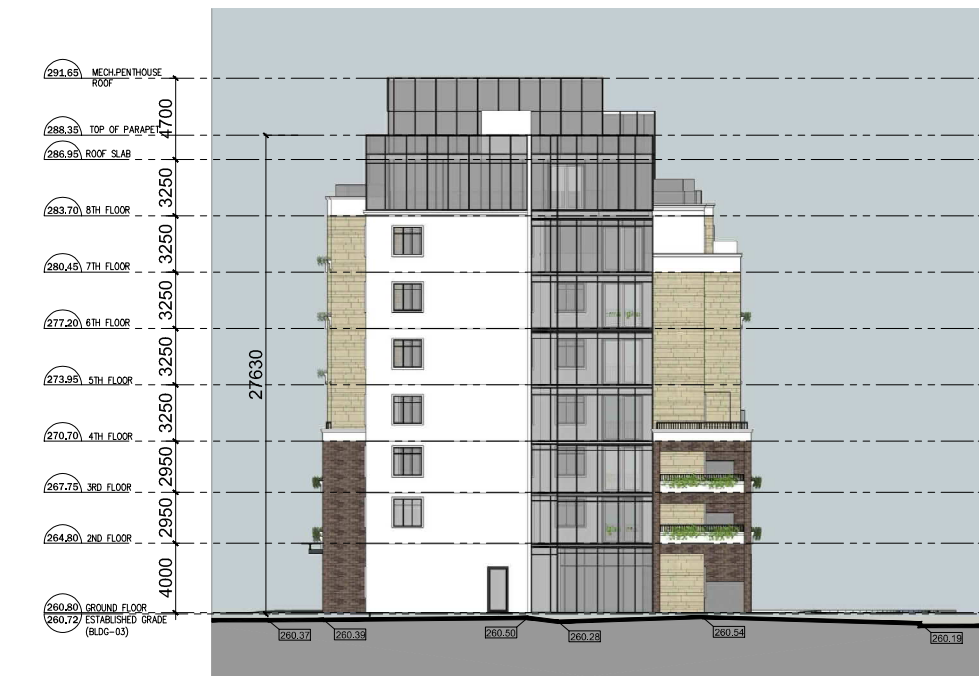
BUILDING - 3 _ EAST ELEVATION _ SCALE 1:200



BUILDING - 3 _ NORTH ELEVATION _ SCALE 1:200



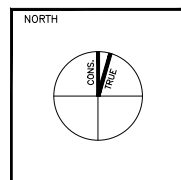
BUILDING - 3 _ WEST ELEVATION _ SCALE 1:200



BUILDING - 3 _ SOUTH ELEVATION _ SCALE 1:200

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NO.	DATE	DESCRIPTION	BY	CHECKED



1 ROSETTA STREET
GEORGETOWN, ON

DRAWING TITLE
ELEVATIONS (BUILDING-3)

Scale:
1/200

Date:
MAR.08, 2022

Project No.
17127

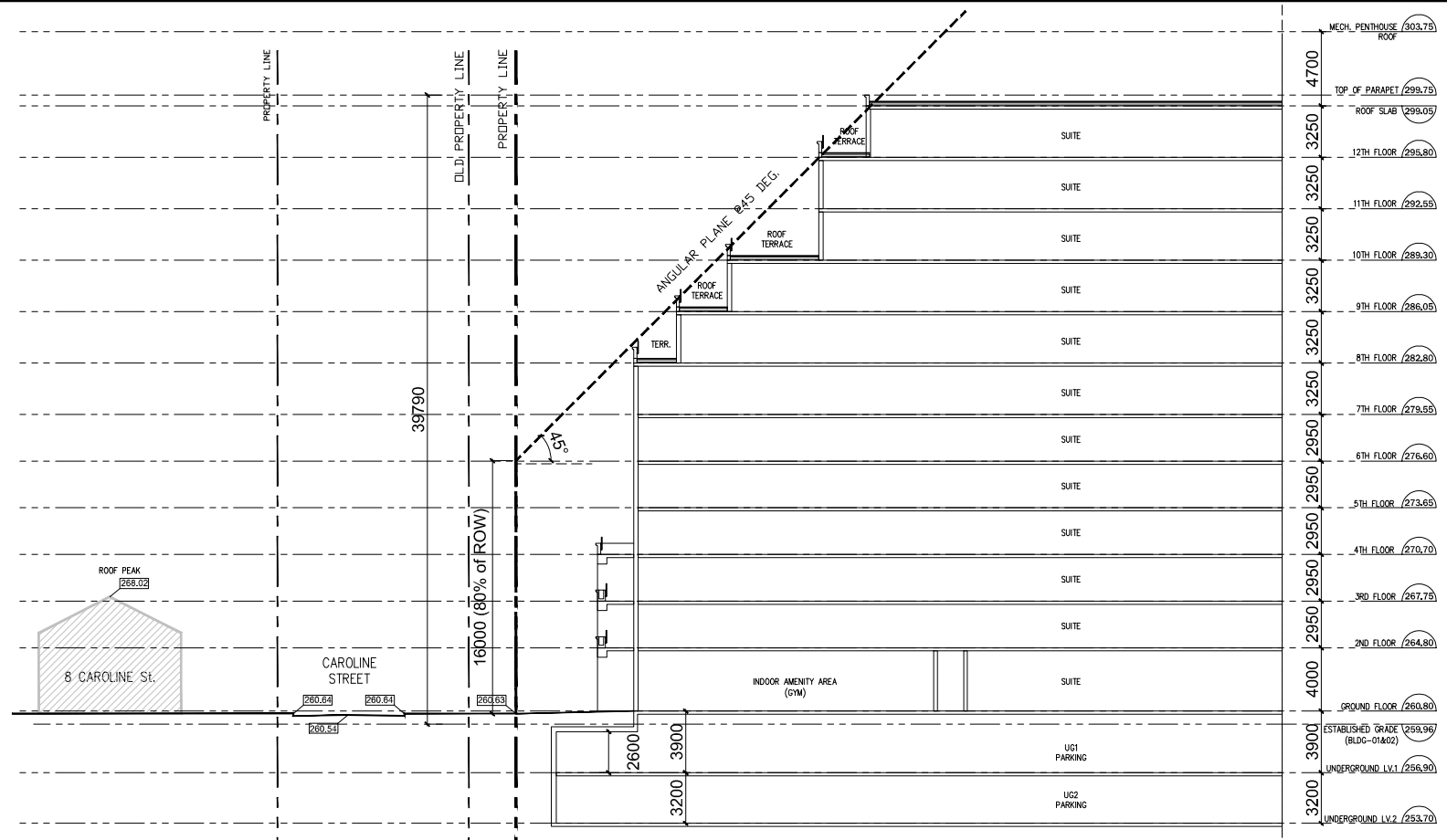
Drawn by:
RK

Checked by:
RE

Drawn No.
A303

815 - 4788 YONGE ST. TORONTO
M5N 0G3 CANADA
T: 416-224-6505 F: 416-224-6504





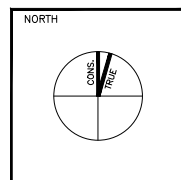
SECTION A _ SCALE 1:200



SECTION B _ SCALE 1:200

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NO.	DATE	BY	CHKD.	APP.	DESCRIPTION



**1 ROSETTA STREET
GEORGETOWN, ON**

DRAWING TITLE
SECTIONS

Scale:
1/200

Date:
MAR.08, 2022

Project No.
17127

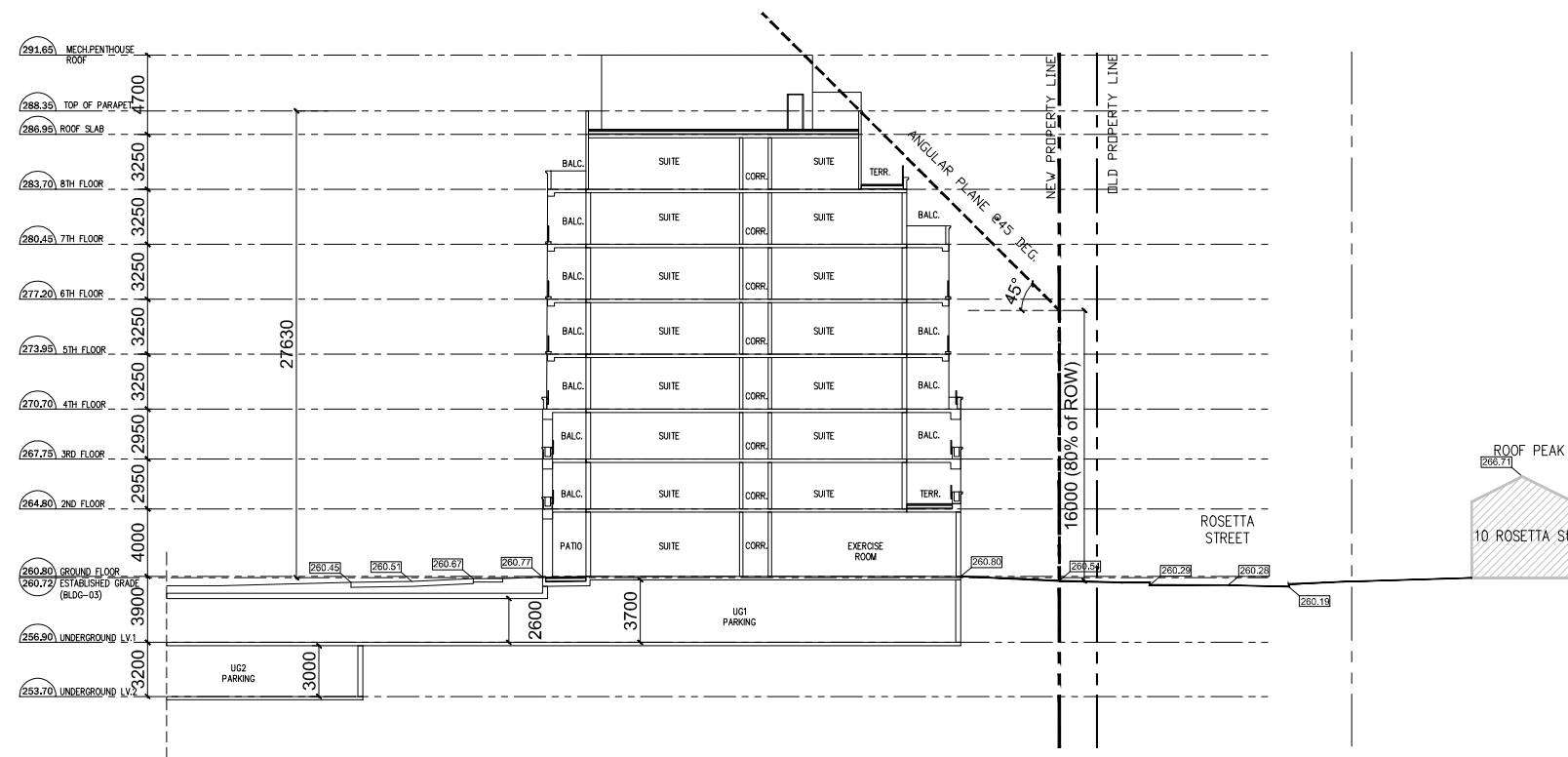
Drawn by:
RK

Checked by:
RE

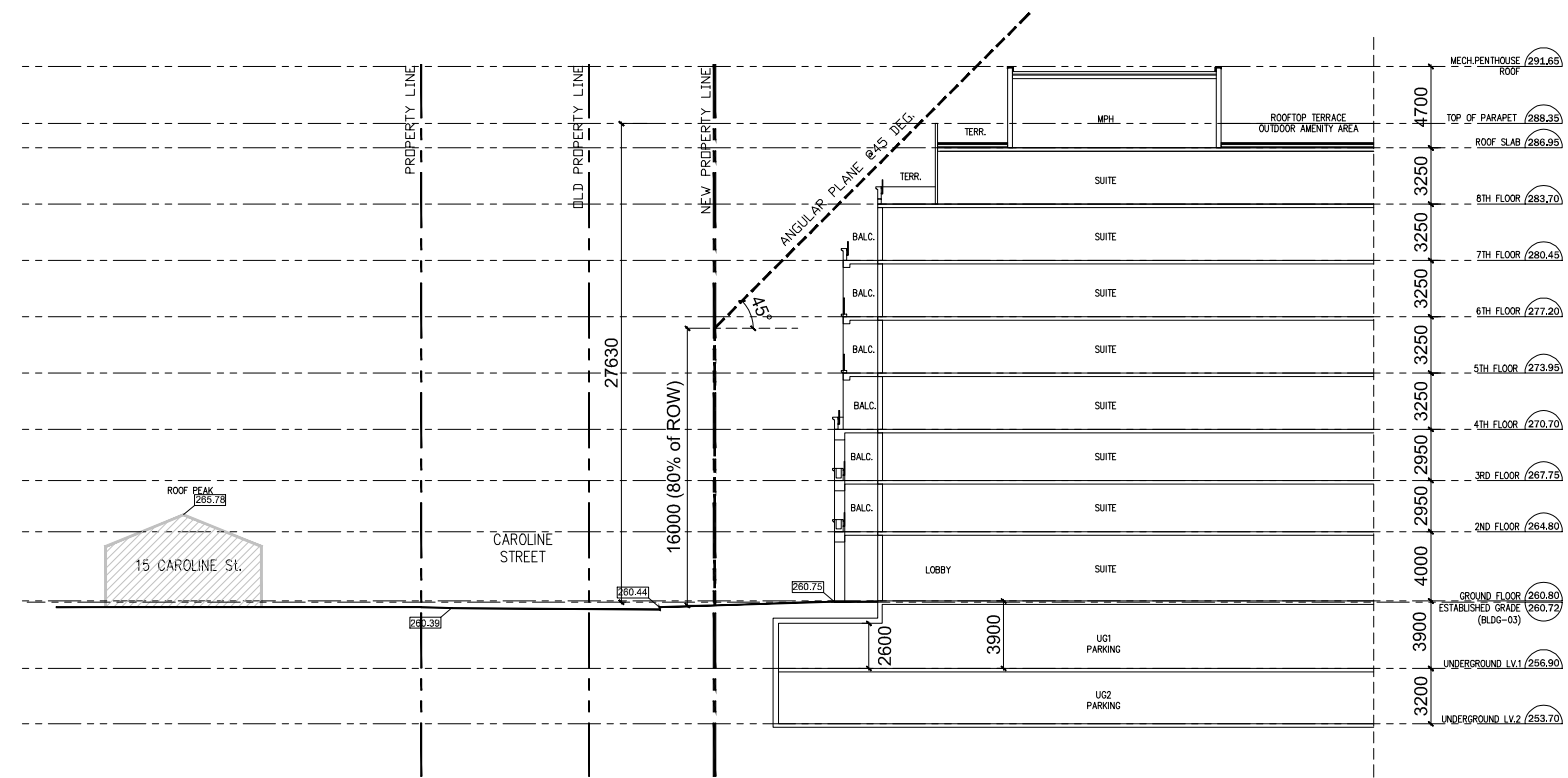
Drawn No.
A401

813 - 4788 YONGE ST. TORONTO
M5N 0G3
T: 416-524-4505 F: 416-524-4504





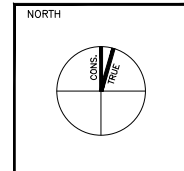
SECTION C _ SCALE 1:200



SECTION D _ SCALE 1:200

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NO.	DATE	DESCRIPTION



1 ROSETTA STREET
GEORGETOWN, ON

DRAWING TITLE
SECTIONS

Scale:
1/200

Date:
MAR.08, 2022

Project No.
17127

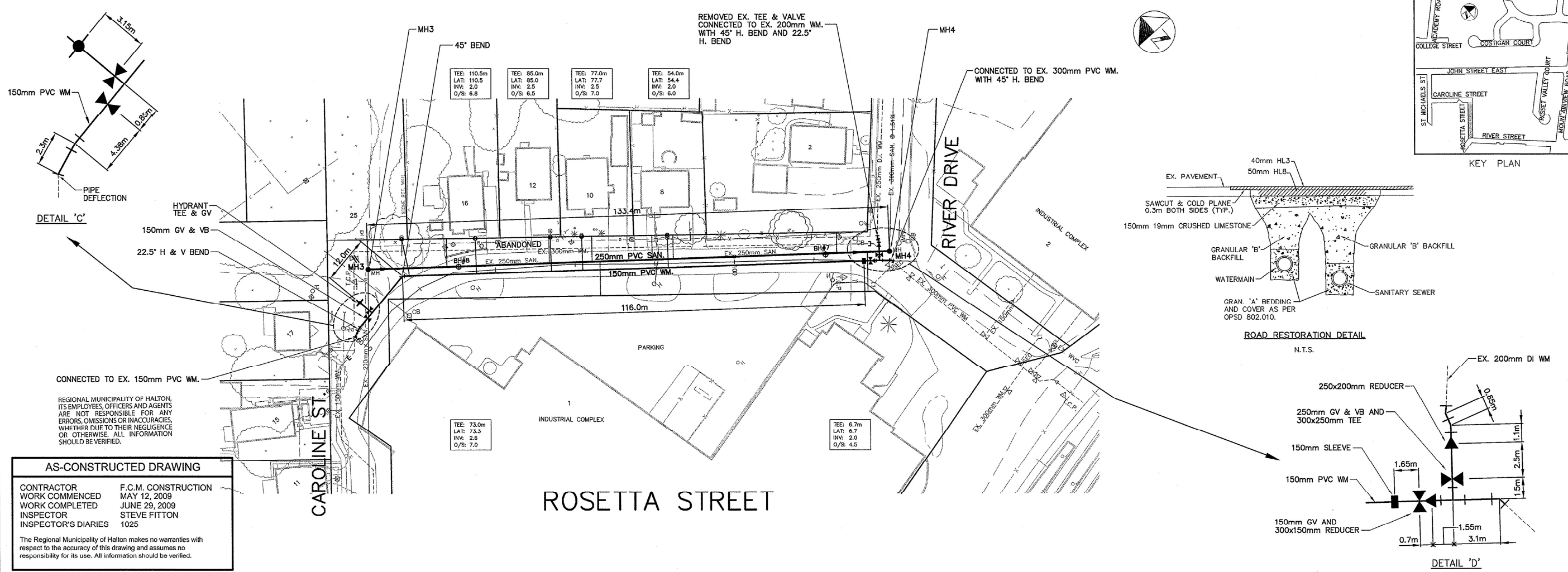
Drawn by:
RK

Checked by:
RE

Drawing No.
A402

813 - 4788 YONGE ST. TORONTO
M5N 0G3
T: 416-224-4505 F: 416-224-0504

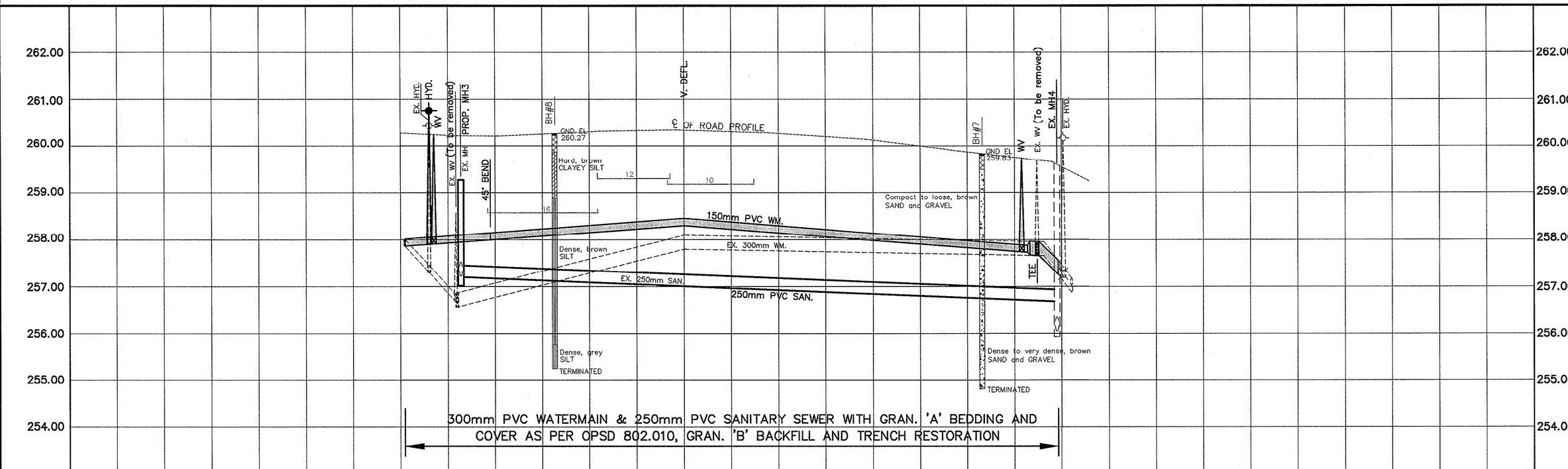
ICON
ARCHITECTS



AS-CONSTRUCTED DRAWING

CONTRACTOR: F.C.M. CONSTRUCTION
 WORK COMMENCED: MAY 12, 2009
 WORK COMPLETED: JUNE 29, 2009
 INSPECTOR: STEVE FITTON
 INSPECTOR'S DIARIES: 1025

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NO	Date	By	REVISIONS	MANU CAD
1	01/05/10	RWB	AS CONSTRUCTED	X
Design	J.P.P.	Ch'kd		Date
Drawn	H.H./L.C.	Ch'kd		JUNE 2008
Scale	Horiz. 1:500	Vert. 1:50		References
APPROVALS				Field Notes
Municipal				Stamp
Regional				
Director, Engineering Services				
Manager, Design Services				



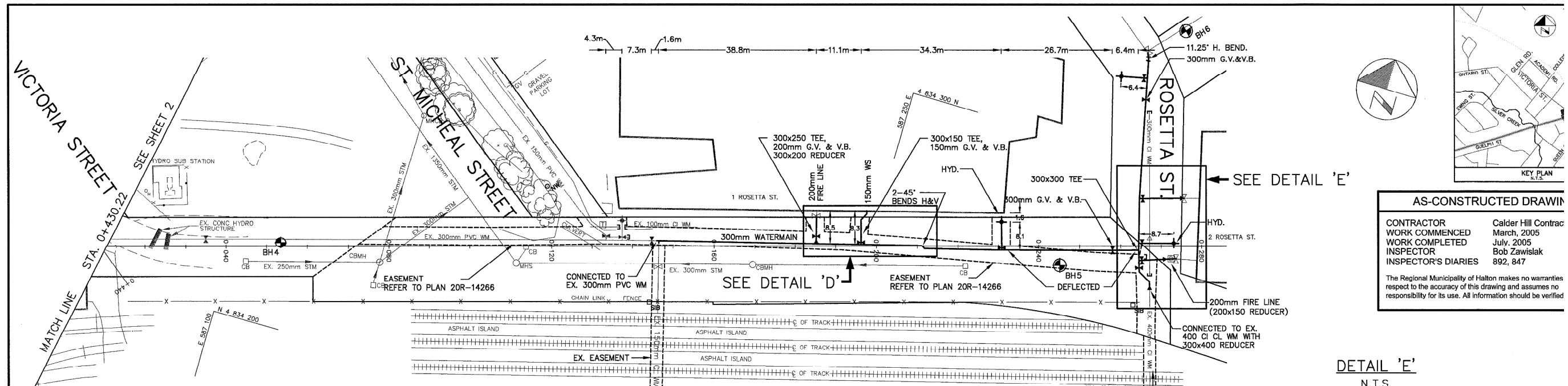
⊕ ELEVATIONS	262.00	261.00	260.00	259.00	258.00	257.00	256.00	255.00	254.00	⊕ ELEVATIONS
PROP. WATERMAIN	257.864	257.22	257.20	257.20	257.20	257.20	257.20	257.20	257.20	PROP. WATERMAIN
PROP. SANITARY SEWER	257.22	257.20	257.20	257.20	257.20	257.20	257.20	257.20	257.20	PROP. SANITARY SEWER
CHAINAGE	0+000	0+020	0+040	0+060	0+080	0+100	0+120	0+140	0+140	CHAINAGE

WATERMAIN & SANITARY SEWER REPLACEMENT ON ROSETTA STREET GEORGETOWN FROM CAROLINE ST. TO RIVER DR.

Consultant File NO: **A-01050**

CONTRACT NO (PR-2450): **WS-2328-08** Drawing NO: **3**
 SHEET **3** OF **3**

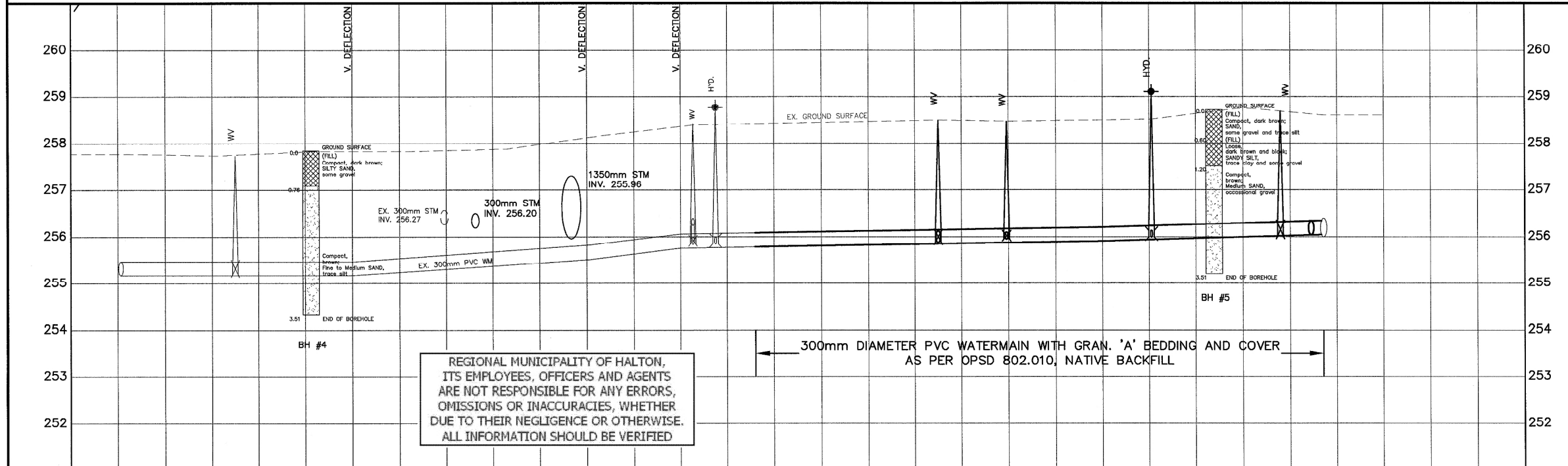
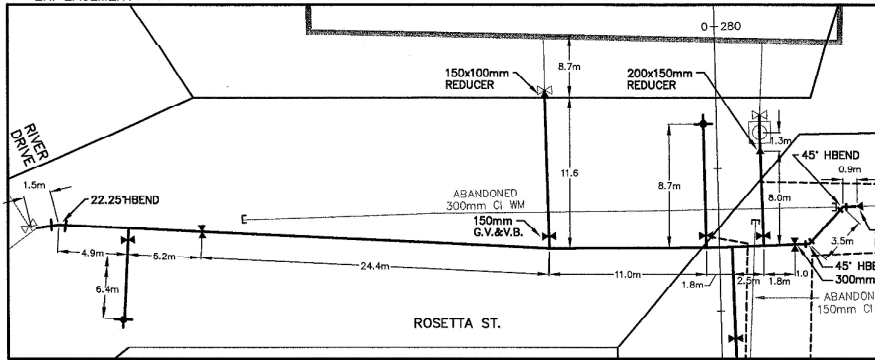
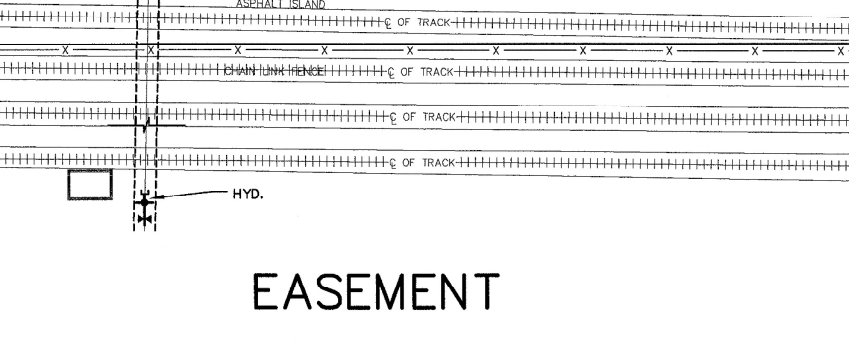
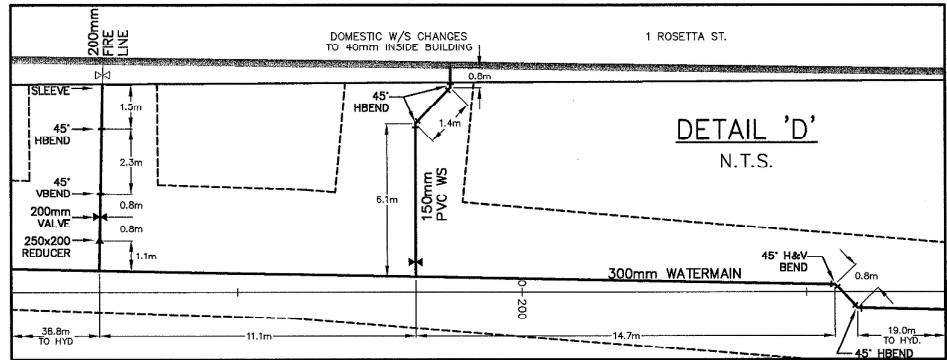
WS-2328-08 3 of 3 A-01050



AS-CONSTRUCTED DRAWING

CONTRACTOR: Calder Hill Contract
 WORK COMMENCED: March, 2005
 WORK COMPLETED: July, 2005
 INSPECTOR: Bob Zawislak
 INSPECTOR'S DIARIES: 892, 847

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NO	Date	By	REVISIONS
1	07/30/07	DMS	"AS CONSTRUCTED"

Design: M.B. Ch'kd
 Drawn: D.M.S. Ch'kd

Scale: HOR. 1:500, VERT. 1:50

APPROVALS: Municipal, Regional (Director, Engineering Services; Manager, Design Services)



STATIONS	ELEVATIONS	PROPOSED WATERMAIN INVERTS	PROPOSED WATERMAIN INVERTS
0+035.0	257.77		
0+040	257.79		
0+060	257.84		
0+080	257.89		
0+100	257.98		
0+120	258.25		
0+140	258.41		
0+160	258.45		
0+180	258.50		
0+185.0	258.48		
0+196.3	258.51		
0+200	258.64		
0+220	258.51		
0+230.5	258.64		
0+240	258.68		
0+260	258.68		
0+267.1	258.61		
0+272.58	256.05		
0+280	256.61		

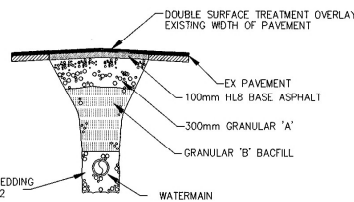
TITLE: WATERMAIN REPLACEMENT & EASEMENT GEORGETOWN FROM VICTORIA ST. TO ROSETTA

Consultant File No: WS-2095-05
 Drawing No: 30
 SHEET 3 of 6

G-3096 PR-2095-05 3 of 6

DRIVEWAY RESTORATION REQUIREMENT

50 mm H3 AT 97% COMPACTION
 150mm GRANULAR 'A' AT 100% COMPACTION
 GRANULAR 'B' BACKFILL AT 100% COMPACTION

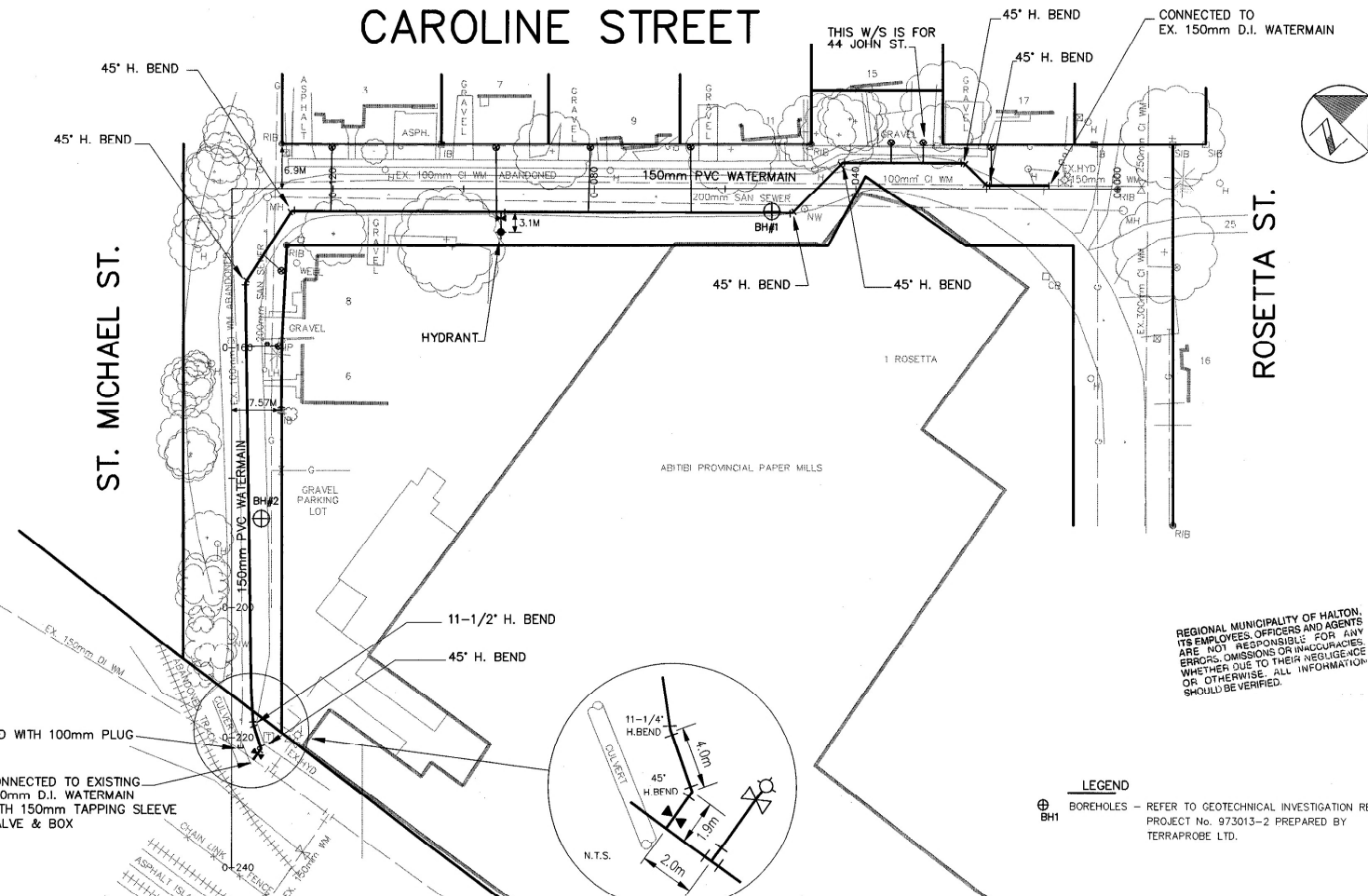


ROAD RESTORATION DETAIL
 N.T.S.

NOTE: ASPHALT RESTORATION SHALL MATCH EXISTING ASPHALT DEPTH OR FOLLOW TYPICAL RESTORATION DETAIL, WHICHEVER IS GREATER.

RAILWAY STATION & YARD
TORONTO AREA TRANSIT
OPERATING AUTHORITY

CAROLINE STREET

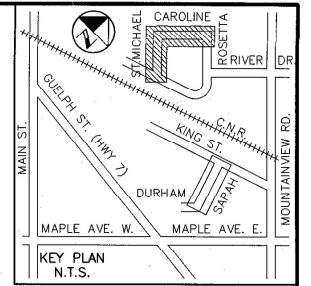


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 ITS EMPLOYEES, OFFICERS AND AGENTS
 ARE NOT RESPONSIBLE FOR ANY
 ERRORS, OMISSIONS OR INACCURACIES
 WHETHER DUE TO THEIR NEGLIGENCE
 OR OTHERWISE. ALL INFORMATION
 SHOULD BE VERIFIED.

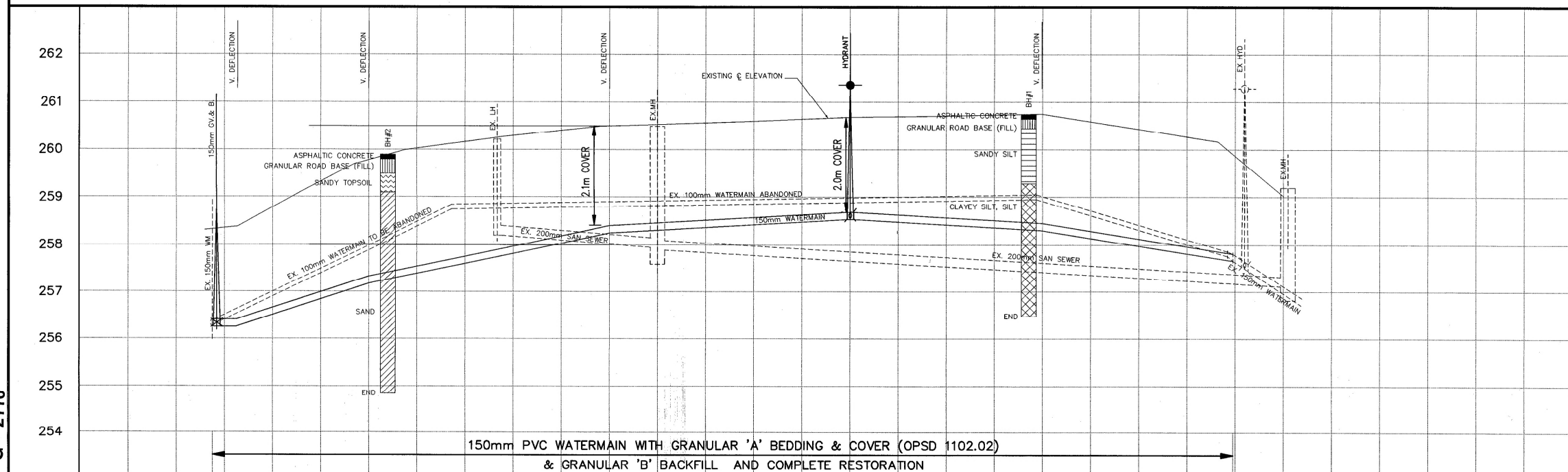
LEGEND
 BH#1 BOREHOLES - REFER TO GEOTECHNICAL INVESTIGATION REPORTS PROJECT No. 973013-2 PREPARED BY TERRAPROBE LTD.

- GENERAL NOTES**
- All dimensions are in Metres unless otherwise specified.
 - All Watermain installation shall conform to the latest revisions of the Ontario Provincial Standard Drawings (OPSD) and specifications (OPSS) as amended by the Regional Municipality of Halton.
 - The locations of all existing Watermain, Sanitary Sewer, Utilities and Service Laterals are approximate. The Contractor must verify the location, size and elevation in the field prior to construction.
 - Watermain can be either Ductile Iron Pressure Class 350 as per AWWA C-150 or PVC SDR-18 CL-150 as per AWWA C-900.
 - All existing water services to be replaced with 20mm diameter copper for residential dwellings and 25mm diameter copper for industrial and commercial premises as per OPSD 1104.01 unless otherwise noted. New services are to be replaced from the main to the property line with a new curb stop and service box at the property line, unless otherwise noted.
 - The Contractor shall provide all temporary caps, plugs and blow offs required for testing the new watermain.
 - Maximum allowable pipe joint deflection of the watermain shall be 70% of the Manufacturer's specifications.
 - Corrosion protection is required for all metallic pipe, valves, fittings services and hydrants. Use either cathodic protection (zinc anode) as per the details in the specification or 3mm medium density Polyethylene encasement as per AWWA C-105.
 - Existing valve boxes to be removed are to have granular backfill and complete restoration. Existing valves in valve chambers to be removed are to have the chamber broken down to 1.0 metres below final grade backfilled with granular to subgrade and complete restoration. All existing hydrants to be removed are to be returned to Regional Stores 2316 South Service Road, Oakville or 1600 Steeles Ave. Milton.
 - If hydrant exceeds 1.7M, a hydrant that can be raised from the bottom without increasing the rod length must be used.
 - Regional Municipality of Halton Approved Mechanical Restraints shall be used on all standard bends, valves, fittings and hydrants. Refer to table below for length of pipe to be restrained on both sides of fittings.

RESTRAINED LENGTHS FOR HORIZONTAL BENDS FOR 150mm DIA. WM				
45 DEG. H. BEND	22.5 DEG. H. BEND	11.25 DEG. H. BEND	TEE & REDUCER	GATE VALVE & HYDRANT
2.0M	1.0M	0.5M	10.0M	11.0M



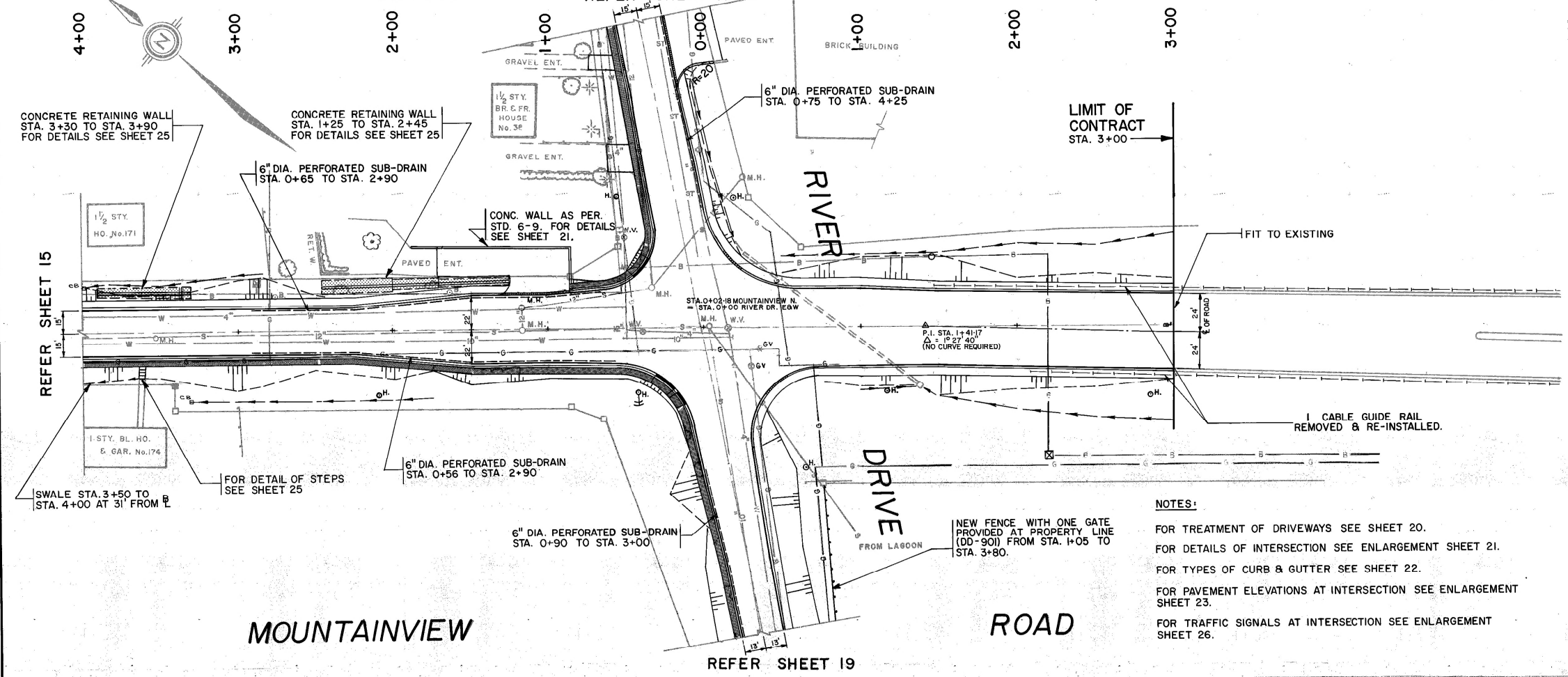
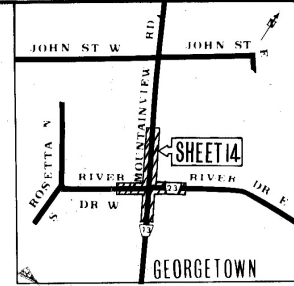
G - 2110



ELEVATIONS	256.35	259.32	260.02	260.29	260.50	260.55	260.64	260.72	260.75	260.59	260.27	257.66	256.02	ELEVATIONS									
WATERMAIN INVERTS	256.23	256.25	257.17	258.25	258.25	258.25	258.53	258.53	258.30	257.66	257.66	257.66	257.66	WATERMAIN INVERTS									
CHAINAGE	0+240	0+222.5	0+220	0+217.5	0+200	0+190	0+180	0+160	0+140	0+120	0+100	0+090	0+080	0+070	0+060	0+050	0+040	0+030	0+020	0+010.5	0+000	CHAINAGE	
	5.0M @ 0.0%		27.5M @ 3.35%		50.0M @ 1.74%				47.0M @ 0.60%			40.0M @ 0.57%		39.5M @ 1.62%									

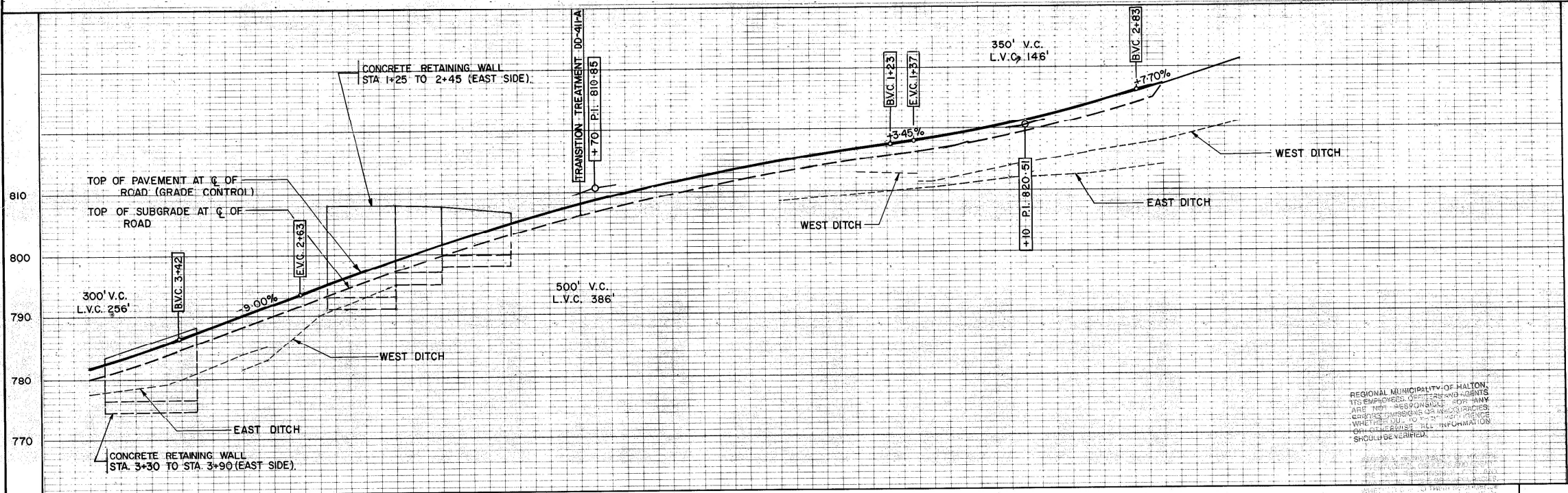
262														262
261	17/03/99	WVC	AS CONSTRUCTED											261
	NO	Date	By	REVISIONS		MANU CAD								
	Design	BW/WVC	Ch'kd											260
	Drawn	WVC	Ch'kd											
	Scale	Horiz.	10	5	0	10								259
		Vert.	1:500	1:500	1:500	1:500								
														258
	APPROVALS			Field Notes		REGION BOOK								257
	Municipal													256
	Regional			Stamp										255
	Director, Design and Construction Services													254
	Manager, Design Services													
	AS CONSTRUCTED													
	Halton													
	TITLE													
	150mm WATERMAIN REPLACEMENT ON CAROLINE ST. & ST. MICHAEL ST. In The Town Of HALTON HILLS (Georgetown) ROSETTA ST. TO C.N.R. TRACKS													
	Consultant File NO			Regional Drawing NO		G - 2110								
	CONTRACT NO			Drawing NO		W-1676-97								
				SHEET		1 OF 4								

REGION OF REFER SHEET 17 HALTON HILLS (GEORGETOWN)



NOTE:
FOR AS CONSTRUCTED WATERMAIN AND SANITARY AND STORM SEWERS SEE SHEET 8.

- NOTES:
- FOR TREATMENT OF DRIVEWAYS SEE SHEET 20.
 - FOR DETAILS OF INTERSECTION SEE ENLARGEMENT SHEET 21.
 - FOR TYPES OF CURB & GUTTER SEE SHEET 22.
 - FOR PAVEMENT ELEVATIONS AT INTERSECTION SEE ENLARGEMENT SHEET 23.
 - FOR TRAFFIC SIGNALS AT INTERSECTION SEE ENLARGEMENT SHEET 26.



REGIONAL MUNICIPALITY OF HALTON, ITS EMPLOYEES OR AGENTS ARE NOT RESPONSIBLE FOR ANY CONSTRUCTION OR PERFORMANCE WHETHER OR NOT INFORMATION OTHERWISE ALL INFORMATION SHOULD BE VERIFIED.

830	26/4/82	M.J.H.	AS CONSTRUCTED.
REVISIONS			
820	Design	D.R.K.	Checked
820	Drawn	M.J.H.	Checked
810	APPROVALS		FIELD NOTES
800	Regional		STAMP

REGIONAL MUNICIPALITY OF HALTON
PUBLIC WORKS DEPARTMENT

TITLE
MOUNTAINVIEW ROAD RECONSTRUCTION
IN THE TOWN OF HALTON HILLS
(GEORGETOWN)

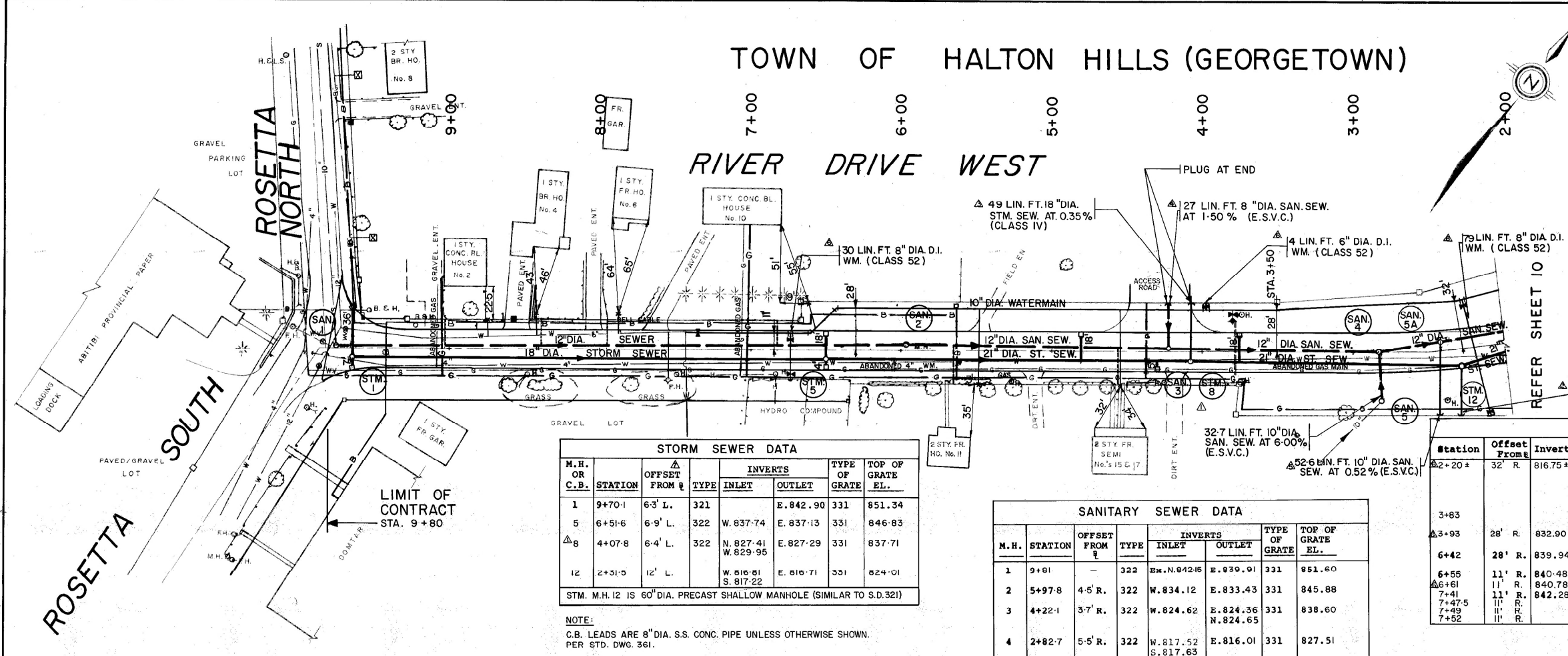
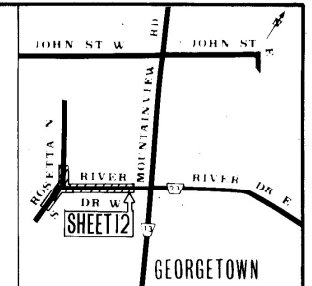
GRADING & PAVEMENT
ELEVATION
STA. 3+50S. TO STA. 4+00N. MOUNTAINVIEW ROAD
MUNICIPAL DRAWING NO. G-866

CONTRACT NO. R-023-81
SHEET 14 OF 36

4+00	3+50	3+00	2+50	2+00	1+50	1+00	0+50	0+00	0+50	1+00	1+50	2+00	2+50	3+00	3+50
781.82	785.66	790.15	794.64	798.86	802.73	806.24	809.39	812.17	814.61	816.68	818.47	820.74	823.75	827.44	
TOP OF PAVEMENT ELEVATION GRADE CONTROL															
CHAINAGE															

TOWN OF HALTON HILLS (GEORGETOWN)

RIVER DRIVE WEST



M.H. OR C.B.	STATION	OFFSET FROM	TYPE	INVERT	OUTLET	TYPE OF GRATE	TOP OF GRATE EL.
1	9+70-1	6'-3" L.	321	E. 842.90	E. 842.90	331	851.34
5	6+51-6	6'-9" L.	322	W. 837-74	E. 837-13	331	846-83
8	4+07-8	6'-4" L.	322	N. 827-41 W. 829-95	E. 827-29	331	837-71
12	2+31-0	12' L.		W. 816-81 S. 817-22	E. 816-71	331	824-01

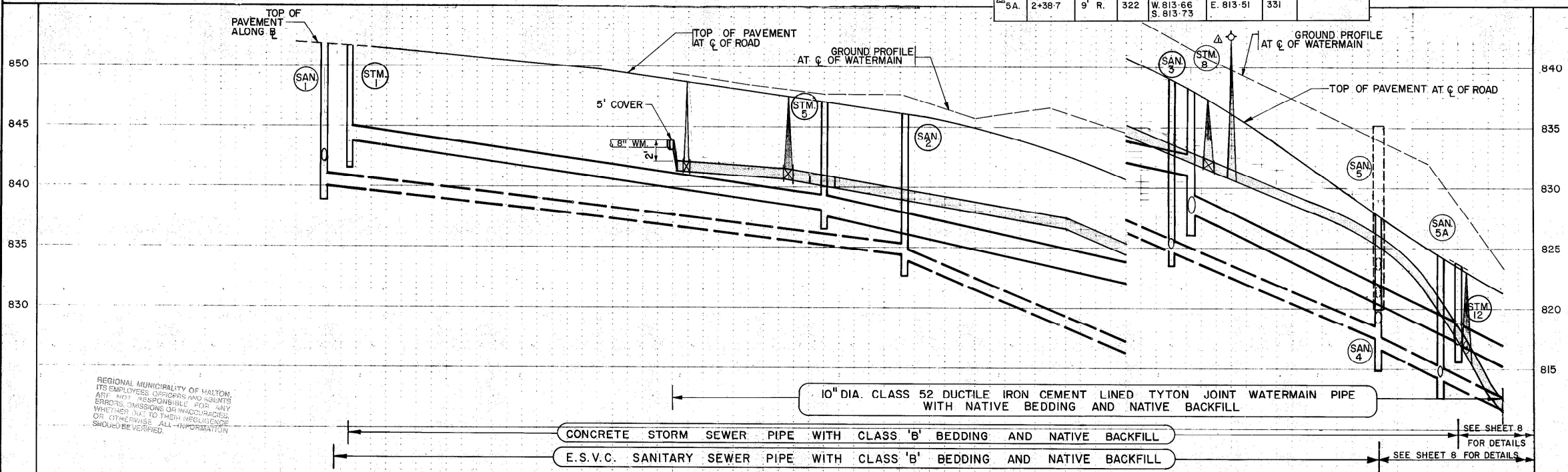
STM. M.H. 12 IS 60" DIA. PRECAST SHALLOW MANHOLE (SIMILAR TO S.D.321)

M.H.	STATION	OFFSET FROM	TYPE	INVERT	OUTLET	TYPE OF GRATE	TOP OF GRATE EL.
1	9+81		322	Ex. N. 842-45 E. 839-91	E. 839-91	331	851.60
2	5+97-8	4'-5" R.	322	W. 834-12	E. 833-43	331	845.88
3	4+22-1	3'-7" R.	322	W. 824-62	E. 824-36 N. 824-65	331	838.60
4	2+82-7	5'-5" R.	322	W. 817-52 S. 817-63	E. 816-01	331	827.51
5	2+85-7	32-7" L.	322	Ex. S.W. 823-29	N. 821-10	331	831.76
5A	2+38-7	9' R.	322	W. 813-66 S. 813-73	E. 813-51	331	

Station	Offset From	Invert	Remarks	Std. Dwg.
2+20±	32' R.	816.75±	10" x 8" TEE WITH 8" G.V. & V.B. & 8" PLUG	
3+83			10" x 6" Anchor Tee & hyd. complete	441
3+93	28' R.	832.90	10" x 6" TEE WITH 6" G.V. & V.B. & 6" PLUG	
6+42	28' R.	839.94	10" Dia. 45° Horiz. bend	
6+55	11' R.	840-48	10" Dia. 45° Horiz. bend	
6+61	11' R.	840.78	10" x 8" TEE WITH 8" G.V. & V.B. & 8" PLUG	405
7+41	11' R.	842.28	10" G.V. and V.B.	
7+47-5	11' R.		10" Dia. 45° Vert. Bend & Ret. Gland Rings	
7+49	11' R.		10" Dia. 45° Vert. Bend & Ret. Gland Rings	
7+52	11' R.		10" x 8" Reducer & connection to 8" WM.	

HOUSE NO.	STA. @ TEE	STA. @ R.	REMARKS
15 & 17	4+56	4+56	21' OF 6" DIA. PVC
11	5+65	5+65	21' OF 6" DIA. PVC
10	6+82	6+82	20' OF 6" DIA. PVC
6	7+96	-	TRANSFER
4	8+39	-	TRANSFER
2	9+03	-	TRANSFER

NOTE:
C.B. LEADS ARE 8" DIA. S.S. CONC. PIPE UNLESS OTHERWISE SHOWN.
PER STD. DWG. 361.



26/4/82	M.J.H.	AS CONSTRUCTED.
10/7/81		CATCH BASIN OFFSETS CHANGED
		WM. CONN. ADDED AT 2+24, 3+97 & 6+73
		SAN. CONN. @ MH 5A ADDED
		SIZE & SLOPE OF SAN. CONN. CHANGED
		SIZE & SLOPE OF STM. CONN. CHANGED
		STM. CONN. ADDED
		LOCATION OF STM. MH 8 CHANGED
REVISIONS		
No.	Date	By
Design	D.R.K.	Checked
Drawn	M.J.H.	Checked
Scale		DATE
HORIZ. 20' = 1"		MARCH 1980
VERT. 2-5' = 1"		REFERENCES
APPROVALS		
Municipal		FIELD NOTES
Regional		STAMP
		MANAGER OF DESIGN
		DIRECTOR OF PUBLIC WORKS

REGIONAL MUNICIPALITY OF HALTON
PUBLIC WORKS DEPARTMENT

MOUNTAINVIEW ROAD RECONSTRUCTION
IN THE TOWN OF HALTON HILLS
(GEORGETOWN)

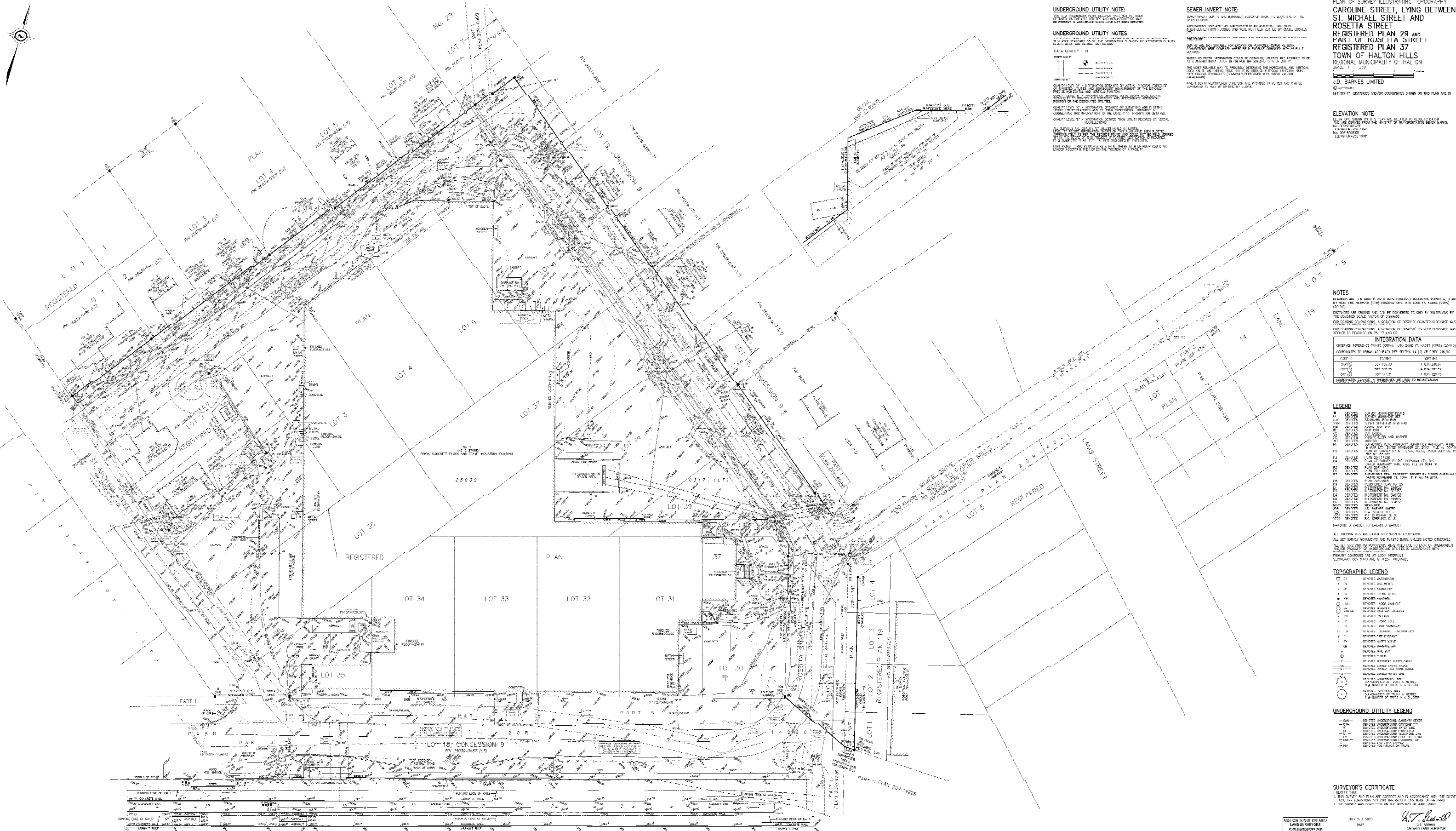
SANITARY & STORM SEWERS & WATERMAIN
STA. 2+00 TO STA. 10+00 RIVER DRIVE WEST

MUNICIPAL DRAWING NO. **G-864**

CONTRACT NO. **R-023-81**

SHEET **12** OF **36**

10+00	+81	+70-1	+50	9+00	+50	8+00	+50	7+00	+50	6+00	+50	5+00	+90	+50	+22-1	+07-8	4+00	+50	3+00	+82-1	+50	+38-7	+31-5	2+00
267 LIN. FT.																								
310 LIN. FT. (PIPE DEFLECTED AT JOINTS)																								
318-5 LIN. FT. OF 18" DIA. CLASS IV PIPE AT 1-62%												243-8 LIN. FT. OF 21" DIA. CLASS IV PIPE AT 2-94%												
384-3 LIN. FT. OF 12" DIA. PIPE AT 1-51%												175-7 LIN. FT. OF 12" DIA. PIPE AT 5-01%												
175-6 LIN. FT. OF 21" DIA. CLASS IV PIPE AT 5-97%												138-5 LIN. FT. OF 12" DIA. PIPE AT 4-94%												
242-8 LIN. FT. OF 21" DIA. CLASS IV PIPE AT 5-83%												238 LIN. FT. OF 12" DIA. PIPE AT 5-88%												
CHAINAGE																								



UNDERGROUND UTILITY NOTE:
THIS IS A PRELIMINARY PLAN. RECORDS MAY NOT BE UP TO DATE. IN CASE OF DISCREPANCY BETWEEN THIS PLAN AND RECORDS, THE RECORDS SHALL PREVAIL.
IF ANY UNDERGROUND UTILITIES ARE FOUND TO BE DIFFERENT FROM THOSE SHOWN ON THIS PLAN, THE SURVEYOR SHALL BE ADVISED IN WRITING.
ALL UTILITIES ARE SHOWN AS LOCATED BY THE SURVEYOR OR OBTAINED FROM UTILITY RECORDS OR GENERAL DATA QUANTITY 10.

SEWER INVERT NOTE:
SEWER INVERTS ARE MANUALLY MEASURED FROM 0.00 M (0.00 FT) TO THE FINISH FLOOR LEVEL.
INVERTS ARE SHOWN AS LOCATED BY THE SURVEYOR OR OBTAINED FROM UTILITY RECORDS OR GENERAL DATA QUANTITY 10.
WHERE NO DEPTH INFORMATION COULD BE OBTAINED, UTILITIES ARE ASSUMED TO BE AT A FINISH FLOOR LEVEL OF 0.00 M (0.00 FT).
THE MOST RELIABLE WAY TO PRECISELY DETERMINE THE HORIZONTAL AND VERTICAL LOCATION OF AN UNDERGROUND UTILITY IS BY EXCAVATION. EXCAVATION SHOULD BE CONDUCTED TO VERIFY THE LOCATION AND DEPTH OF ANY UTILITY SHOWN ON THIS PLAN.

PLAN OF SURVEY ILLUSTRATING TOPOGRAPHY
CAROLINE STREET, LYING BETWEEN
ST. MICHAEL STREET AND
ROSETTA STREET
REGISTERED PLAN 29 AND
PART OF ROSETTA STREET
REGISTERED PLAN 37
TOWN OF HALTON HILLS
REGIONAL MUNICIPALITY OF HALTON
DATE: 17-08-2024
J.D. BARNES LIMITED
METRIC: DIMENSIONS AND COORDINATES SHOWN ON THIS PLAN ARE IN METERS

ELEVATION NOTE:
ELEVATIONS SHOWN ON THIS PLAN ARE RELATED TO GEODETIC DATUM 1984 (NAD 83) AND ARE DERIVED FROM THE HORIZONTAL AND VERTICAL CONTROL DATA OF THE REGIONAL MUNICIPALITY OF HALTON.

INTEGRATION DATA

POINT NO.	EASTING	NORTHING
01	487 138.18	4 831 218.87
02	487 200.00	4 834 080.00
03	487 261.82	4 831 120.70

- NOTES**
- 1. THIS PLAN IS A PRELIMINARY PLAN. RECORDS MAY NOT BE UP TO DATE. IN CASE OF DISCREPANCY BETWEEN THIS PLAN AND RECORDS, THE RECORDS SHALL PREVAIL.
 - 2. ALL DIMENSIONS ARE IN METERS AND DECIMALS THEREOF.
 - 3. DISTANCES ARE GIVEN AND CAN BE CONVERTED TO CHAIN BY MULTIPLYING BY THE CONVERSION FACTOR OF 0.0254.
 - 4. FOR BEARING CONVERSIONS, A DEFINITION OF ANGLE OR COURSE IS CONSIDERED AS BEING APPLIED TO THE RIGHT OF THE POINT TO WHICH THE BEARING IS APPLIED TO BEING ON THE RIGHT AND TO THE LEFT OF THE POINT TO WHICH THE BEARING IS APPLIED TO BEING ON THE LEFT.
 - 5. COORDINATE DATA IS BASED ON THE NORTH AMERICAN DATUM 1983 (NAD 83) AND IS ACCURATE TO WITHIN 0.01 METERS PER SECTION 14 (2) OF THE SURVEY ACT.

- LEGEND**
- 01 CONCRETE CURB
 - 02 CONCRETE CURB WITH FINISH
 - 03 CONCRETE CURB WITH FINISH AND GUTTER
 - 04 CONCRETE CURB WITH FINISH AND GUTTER AND SIDEWALK
 - 05 CONCRETE CURB WITH FINISH AND GUTTER AND SIDEWALK AND DRIVE
 - 06 CONCRETE CURB WITH FINISH AND GUTTER AND SIDEWALK AND DRIVE AND SIDEWALK
 - 07 CONCRETE CURB WITH FINISH AND GUTTER AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK
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- TOPOGRAPHIC LEGEND**
- 01 CONCRETE CURB
 - 02 CONCRETE CURB WITH FINISH
 - 03 CONCRETE CURB WITH FINISH AND GUTTER
 - 04 CONCRETE CURB WITH FINISH AND GUTTER AND SIDEWALK
 - 05 CONCRETE CURB WITH FINISH AND GUTTER AND SIDEWALK AND DRIVE
 - 06 CONCRETE CURB WITH FINISH AND GUTTER AND SIDEWALK AND DRIVE AND SIDEWALK
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 - 17 CONCRETE CURB WITH FINISH AND GUTTER AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK
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 - 19 CONCRETE CURB WITH FINISH AND GUTTER AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK
 - 20 CONCRETE CURB WITH FINISH AND GUTTER AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK

- UNDERGROUND UTILITY LEGEND**
- 01 CONCRETE UNDERGROUND SANITARY SEWER
 - 02 CONCRETE UNDERGROUND SANITARY SEWER WITH FINISH
 - 03 CONCRETE UNDERGROUND SANITARY SEWER WITH FINISH AND GUTTER
 - 04 CONCRETE UNDERGROUND SANITARY SEWER WITH FINISH AND GUTTER AND SIDEWALK
 - 05 CONCRETE UNDERGROUND SANITARY SEWER WITH FINISH AND GUTTER AND SIDEWALK AND DRIVE
 - 06 CONCRETE UNDERGROUND SANITARY SEWER WITH FINISH AND GUTTER AND SIDEWALK AND DRIVE AND SIDEWALK
 - 07 CONCRETE UNDERGROUND SANITARY SEWER WITH FINISH AND GUTTER AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE
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 - 16 CONCRETE UNDERGROUND SANITARY SEWER WITH FINISH AND GUTTER AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK
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 - 19 CONCRETE UNDERGROUND SANITARY SEWER WITH FINISH AND GUTTER AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK
 - 20 CONCRETE UNDERGROUND SANITARY SEWER WITH FINISH AND GUTTER AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK AND DRIVE AND SIDEWALK

BEFORE DIGGING, UNDERGROUND SERVICES SHOULD BE LOCATED ON SITE BY THE INSTALLER. CALLERS.

CAUTION: CALL BEFORE YOU DIG
CALL 3-1-1 OR 905-874-2222 FOR MORE INFORMATION.
IF YOU ARE A CONTRACTOR, CALL 3-1-1 OR 905-874-2222 FOR MORE INFORMATION.
IF YOU ARE A HOMEOWNER, CALL 3-1-1 OR 905-874-2222 FOR MORE INFORMATION.

BEFORE DIGGING, UNDERGROUND SERVICES SHOULD BE LOCATED ON SITE BY THE INSTALLER. CALLERS.

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Appendix B – Water Analysis

Hydrant Flow Test (River Road)
Water Demand, Fire Demand, and Hazen-Williams Calculations



81 Todd Road Suite 202 Georgetown Ont. L7G 4R8

(o) 905-467-5853 (C) 905-971-9956 (e) mark@aquacom.ca

SITE NAME I B I GROUP

TEST DATE TIME MONDAY 14 DECEMBER 2020 @ 1105 AM

SITE ADDRESS 7 RIVER RD, GEORGETOWN, R OF HALTON

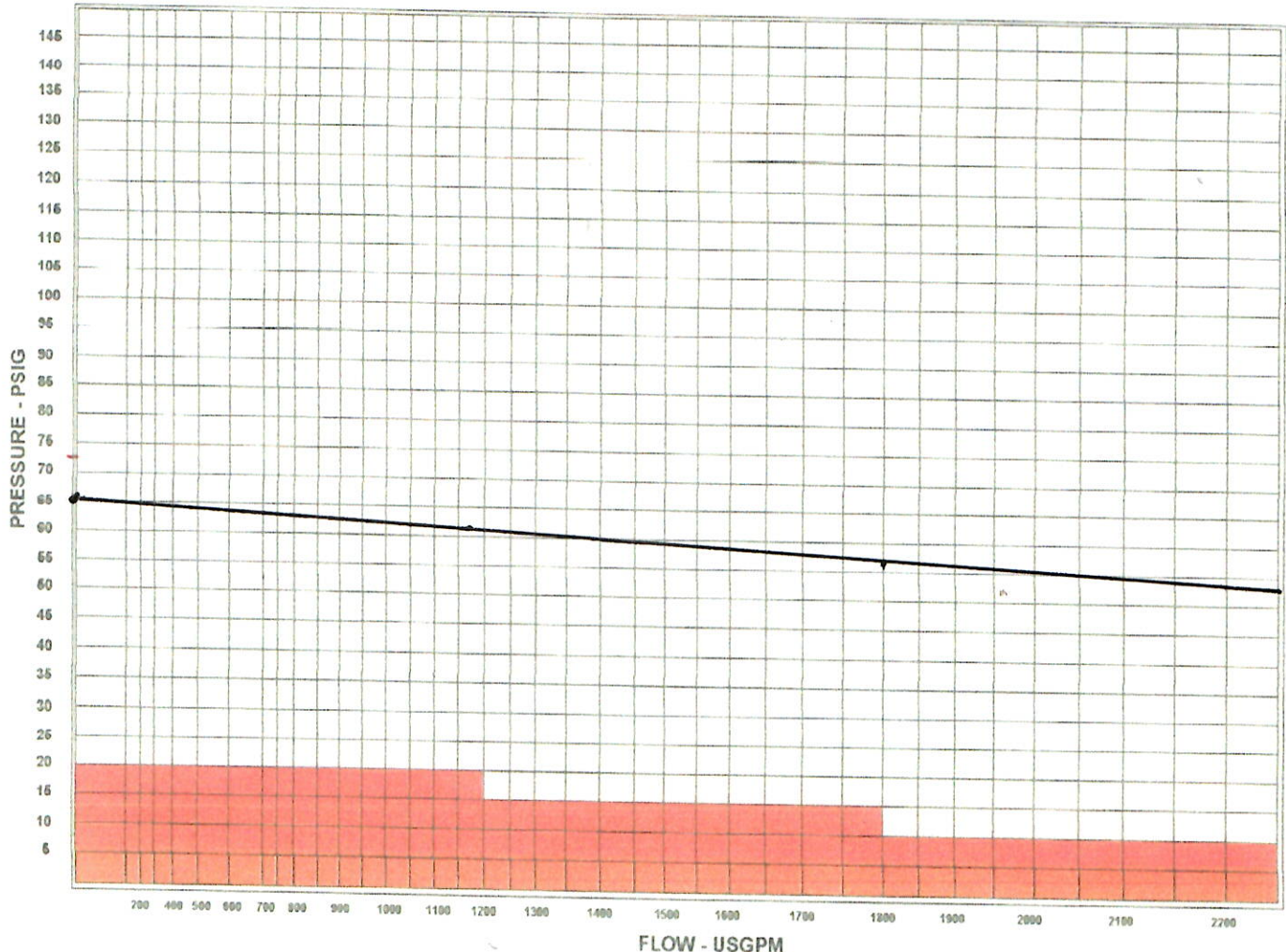
TECHNICIANS G. SUTHERLAND, B. SUTHERLAND

COMMENTS ASSISTANCE FROM RofH OPERATOR

LOCATION OF FLOW HYDRANT
7 RIVER RD

LOCATION OF RESIDUAL HYDRANT
4 DANIEL CT AT RIVER RD

# OUTLETS	SIZE INCHES	PITO PSI	FLOW USGPM	RESIDUAL PSI	STATIC PSI	PIPE DIA. MM
ONE	2.50	45	1151	61	65	
TWO	2.50	29	1798	57		200MM
		THEORETICAL	4569	20	TEST #	ONE
NOZZLE COEFF.		.90				



1 Rosetta Street, Georgetown

The Paper Mill



DOMESTIC DEMAND CALCULATIONS

Project Name: 1 Rosetta Street
 Project Number: 125082
 Date: February 25, 2022
 Designed By: Carly Mason, B.Eng

- Based on the City of Toronto Standards and
- OBC, Part 8 "Sewage Systems", OBC Table 8.2.1.3.A and 8.2.1.3.B
- ADD = 275 L/cap/day for residential uses
- ADD = 250 L/cap/day for commercial uses

Peaking Factors		
Land Use	Peak Hour	Maximum Day
Residential	4.00	2.25

Component	No. Units	Density	Population	ADD (L/s)	(ADDxP.F.) PHD (L/s)	(ADDxP.F.) MDD (L/s)
Residential - 1 Bedroom	490	1.328 pp/unit	651	2.07	8.28	4.66
Residential - 2+ Bedroom	150	1.724 pp/unit	259	0.82	3.29	1.85
	0	0.000 pp/unit	0	0.00	0.00	0.00
Totals			909	2.89	11.58	6.51

FIRE FLOW DEMAND CALCULATIONS

Based on the Water Supply for Public Fire Protection Manual, 1999 by the Fire Underwriters Survey

Step 1: Calculate Fire Flow (based on area)

Construction Coefficient =	0.6	
Largest Floor Area =	3,148	m ²
Floor Above =	3,148	m ²
Floor Below =	3,148	m ²
Area =	4,722	m ²
Fire Flow (F) =	9,000	L/min

F = required fire flow (L/min)

$$F = 220C\sqrt{A}$$

C = coefficient related to type of construction

- 0.6 for fire resistive (fully protected, 3-hr ratings)
- 0.8 for non combustable (i.e. unprotected metal buildings)
- 1.0 for ordinary construction
- 1.5 for wood frame construction

A = total floor area excluding basements 50% below grade

* If vertical openings are inadequately protected, consider two largest two largest adjoining floors plus 50% of each of any floors above up to eight floors.

* If vertical openings are adequately protected (one hour rating), consider largest floor area + 25% of two immediately floors.

Step 2: Adjustment for Building Occupancy (shall not be less than 2000 L/s)

Occupancy Adjustment =	-0.15	
F ₁ = Fire Flow x Adjustment =	7,650	L/min

Non-Combustable	-25%	Free Burning	15%
Limited	-15%	Rapid Burning	25%
Combustable)	No change		

Step 3: Adjust F₁ for Fire Suppression System

Sprinkler Adjustment =	30%	
F ₂ = F ₁ x Adjustment =	2,295	L/min

Automatic Sprinklers (monitored)	-50%
Adequately Designed System	-30%

Step 4: Adjust F₁ for Exposure / Proximity (shall not exceed 75%)

Proximity Adjustment =	30%	(max 75%)
F ₃ = F ₁ x Factor =	2,295	L/min

Separation	Adjustment	Separation	Adjustment
0m to 3m	25%	20.1m to 30m	10%
3.1m to 10m	20%	30.1m to 45m	5%
10.1m to 20m	15%		

Step 5: Calculate Adjusted Fire Flow (shall not be less than 2000 L/min or greater than 45,000 L/min)

F ₁ =	7,650	L/min
- F ₂ =	2,295	L/min
+ F ₃ =	2,295	L/min
Fire Flow =	8,000	L/min
Fire Flow =	133.3	L/s
Total Demand (Fire Flow + MDD) =	139.8	L/s

$$\text{Fire Flow} = F_1 - F_2 + F_3$$

Checks:

Fire Flow greater than 2000 L/min
 Fire Flow less than 45,000 L/min



Project Name: 1 Rosetta Street
 Project Number: 125082
 Date: February 25, 2022
 Designed By: Carly Mason, B.Eng

Hydrant Flow Test Results

Flow (gpm)	Flow (L/s)	Flow (L/min)	Pressure (psi)	Pressure (kPa)
0	0.00	0	65	448
1,151	72.62	4,357	61	421
1,798	113.44	6,806	57	393

(1 gal = 3.785 L)

Residual Pressure at Main

Source: Walski, Thomas M. (2007): Advanced Water Distribution Modeling and Management

$$Q_R = Q_F \times \frac{h_r^{0.54}}{h_f^{0.54}}$$

where: Q_R = flow predicted at desired residual pressure
 Q_F = total flow measured during test
 h_r = pressure drop to desired residual pressure
 h_f = pressure drop to measured during test

	Flow (gpm)	Flow (L/s)	Flow (L/min)	Residual Pressure @ Main	
				(psi)	(kPa)
Domestic Fire	184	11.6	695	64.9	447
	2,217	139.8	8,391	53.2	367

Residual Pressure at Building

$$h_L = \frac{10.675 * L * Q^{1.85}}{C^{1.85} * D^{4.8655}}$$

where: h_L = Pressure Drop (m)
 L = Length of Service (m)
 Q = Flow Rate (m³/s)
 C = Roughness Coefficient
 D = Pipe Diameter (m)

Domestic	
L=	90.0 m
Q=	0.012 m ³ /s
C=	100
D=	150 m
h_L =	0.5 m
h_L =	20.1 in
h_L =	0.7 psi
h_L =	5.0 kPa

Fire	
L=	90.0 m
Q=	0.140 m ³ /s
C=	110
D=	200 m
h_L =	10.6 m
h_L =	418.3 in
h_L =	15.1 psi
h_L =	104.2 kPa

	Flow (gpm)	Flow (L/s)	Flow (L/min)	Residual Pressure @ Bldg.	
				(psi)	(kPa)
Domestic Fire	184	11.6	695	64.2	442
	2,217	139.8	8,391	38.1	263

Residual Pressure (DOMESTIC) at building is greater than 40 psi (275 kPa).
 Residual Pressure (FIRE) at building is greater than 20 psi (140 kPa).

Appendix C – Sanitary Analysis

Sanitary Design Calculations
Sanitary Capacity Review (TMIG)

1 Rosetta Street

The Paper Mill



Pre-development San. Flow - Industrial = **34 L/cd**
 Pre-development San. Flow - Residential = **275 L/cd**
 Post-development San. Flow = **275 L/cd**
 Infiltration (dry weather) = **0.286 L/s/ha**

Mannings= **0.013**
 Minimum flow velocity = **0.6 m/s**
 Maximum flow velocity = **3.0 m/s**

Sanitary Sewer Design Sheet

Project Name: 1 Rosetta Street
 Project Number: 125082
 Date: February 25, 2022
 Designed By: Carly Mason

DESIGN FLOW CALCULATIONS												SEWER DESIGN & ANALYSIS						
Area (ha) Number of Units	Density	Population	Cumulative Area (ha)	Cumulative Population	Kav	Peaking Factor	Sewage Flow (L/s) (1)	Infiltration Flow (L/s) (2)	Industrial Flow (L/s) (3)	Total Flow, Qd (L/s)	Nominal Diameter (mm)	Pipe Slope (%)	Pipe Length (m)	Full Flow Capacity, Qf (L/s)	Full Flow Velocity (m/s)	Actual Velocity V (m/s)	Percent of Full Flow (%)	Notes
Pre-Development																		
Industrial	1.3492 ha	125 pp/ha	169	1.3492	169	0.81	3.39	0.000	0.386	0.067	0.45							
Residential	0.0865 ha	55 pp/ha	5	0.0865	5	0.81	3.61	0.012	0.025	0.000	0.04							
										<i>Total</i>	0.49							
Post-Development																		
Infiltration	1.4356 ha		0.00	1.4356	0	1	3.83	0.000	0.411		0.411							
1 Bedroom	490 Units	1.328 pp/unit	651		651	1	3.83	7.936			7.936							
2+ Bedrooms	150 Units	1.724 pp/unit	259		259	1	3.83	3.157			3.157							
										<i>Total</i>	11.504	250	1.00%	16.5	62.0	1.22	0.93	18.5%

Appendix D – Stormwater Analysis

Stormwater Design Calculations
Pre- and Post-Development Drainage Area Plans

1 Rosetta Drive

The Paper Mill

**Post-Development Runoff Coefficients**

Project Name: 1 Rosetta Drive

Project Number: 125082

Date: May 10, 2022

Designed By: Carly Mason, B.Eng

Pre-Development				
Conventional Roof	8,269	60.9%	0.90	0.55
Green Roof:	0	0.0%	0.50	0.00
Landscaping:	1,002	7.4%	0.25	0.02
Permeable Pavers:	0	0.0%	0.55	0.00
Impervious:	4,301	31.7%	0.90	0.28
Total Area:	13,572	100%		0.85

Total Post-Development				
Conventional Roof	6,271	46.2%	0.90	0.42
Green Roof:	0	0.0%	0.50	0.00
Landscaping:	4,005	29.5%	0.25	0.07
Permeable Pavers:	3,296	24.3%	0.55	0.13
Impervious:		0.0%	0.90	0.00
Total Area:	13,572	100.0%		0.62



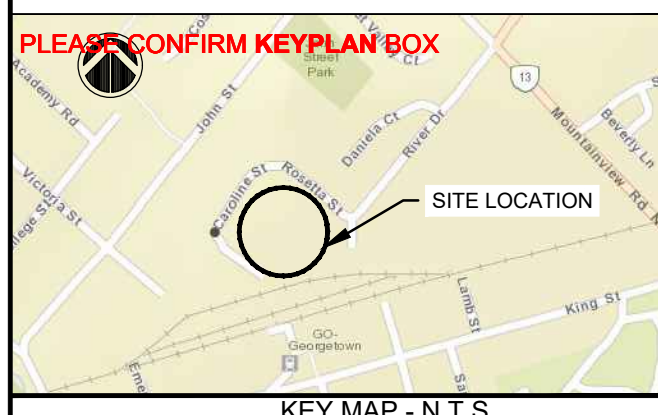
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ISSUES

No.	DESCRIPTION	DATE
1	ZBA SUBMISSION	MAY 12, 2022

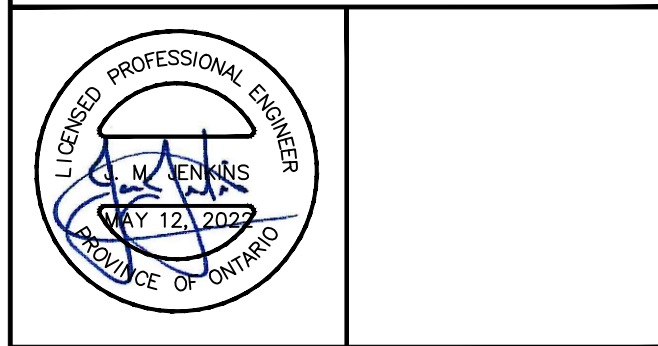
PRELIMINARY



LEGEND

- ORIGINAL GROUND CONTOUR
- EXISTING ELEVATION
- EXISTING DIRECTION OF FLOW
- EXISTING STORM WATER CATCHMENT AREA
- CATCHMENT AREA NUMBER
- RUNOFF COEFFICIENT
- AREA IN HECTARES

SOURCE:
 TOPOGRAPHIC SURVEY INFORMATION OBTAINED FROM J.D. BARNES LTD.
 MILTON, ON
 DRAWING FILE: 17-30-157-02-A
 DATED: JULY 2, 2020
 ELEVATIONS SHOWN ON THIS PLAN ARE RELATED TO GEODETIC DATUM AND ARE DERIVED FROM THE MINISTRY OF TRANSPORTATION
 BENCH MARKS: No. 0011954U598F ELEVATION=258.735m



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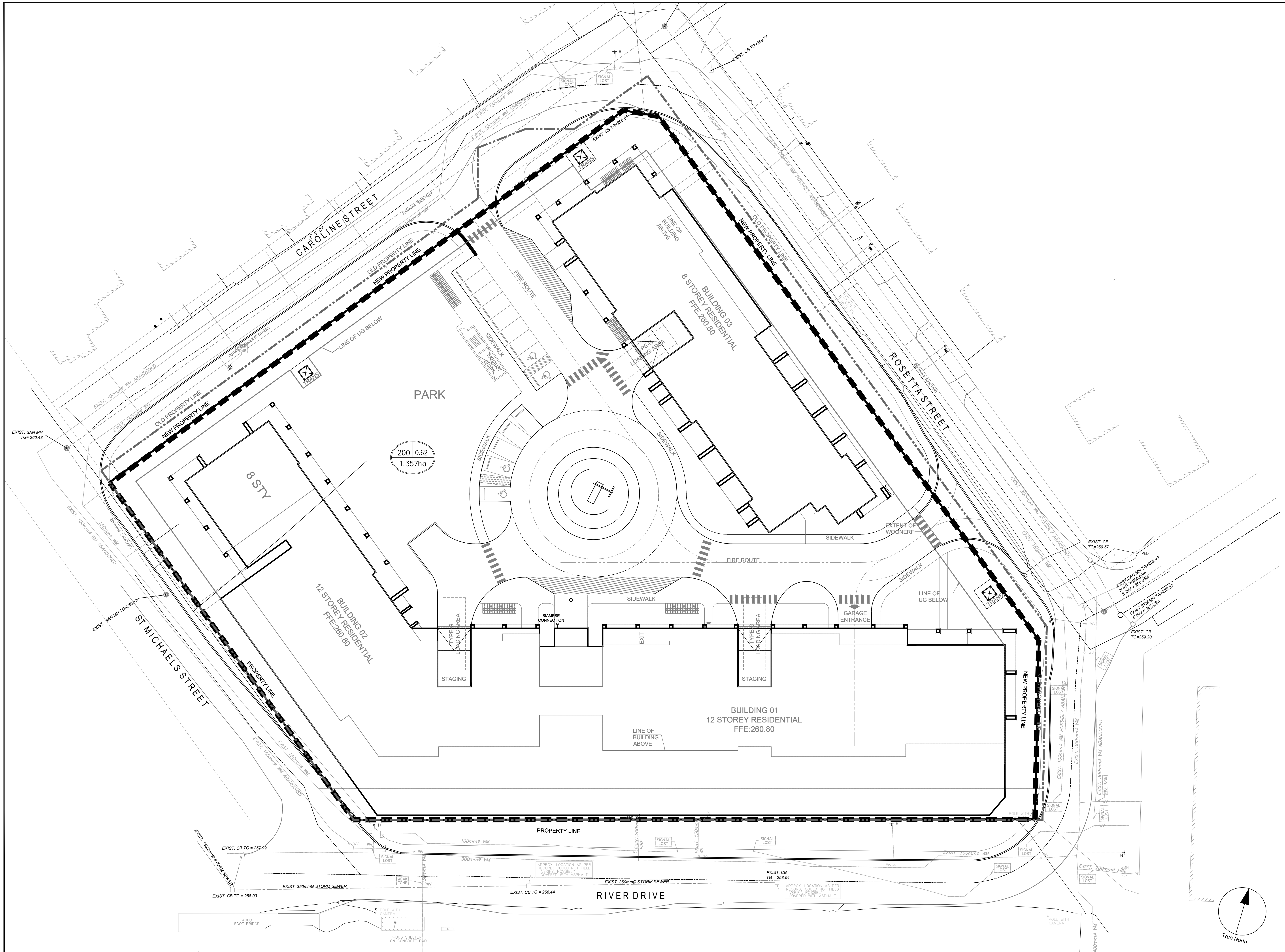
PROJECT
The Paper Mill, 1 Rosetta Street, Georgetown
 1 Rosetta Street, Georgetown
 Township of Halton Hills

PROJECT NO: 125082	SCALE: 1:300
DRAWN BY: KP	CHECKED BY: J. Jenkins
PROJECT MGR: R. Haider	APPROVED BY: J. Jenkins

SHEET TITLE
PRE-DEVELOPMENT STORM CATCHMENT AREA PLAN

SHEET NUMBER 3	ISSUE 1
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 May 11, 2022, 10:26:09 PM by Carly Mason PH0801
 Wednesday, May 11, 2022, 10:26:09 PM by Carly Mason PH0801

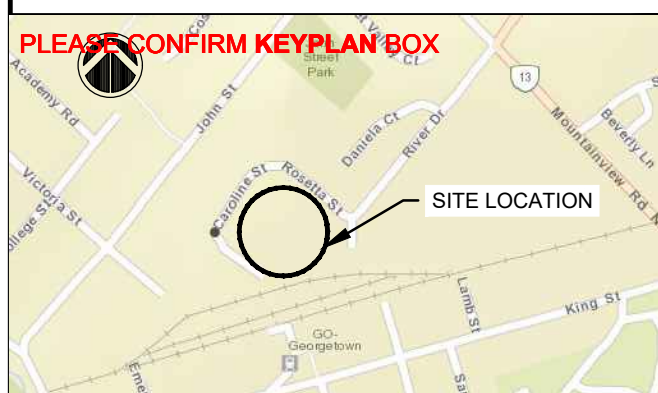


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ISSUES		
No.	DESCRIPTION	DATE
1	ZBA SUBMISSION	MAY 12, 2022

PRELIMINARY



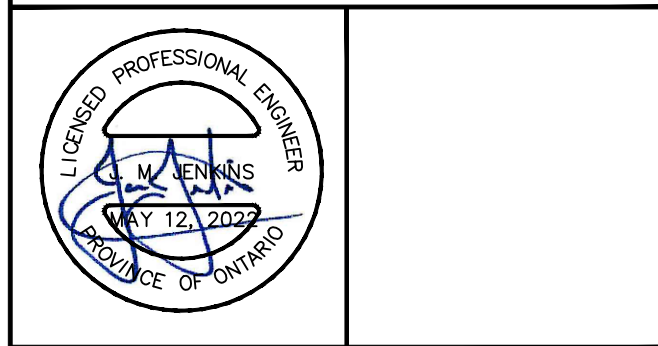
KEY MAP - N.T.S.

LEGEND

- ORIGINAL GROUND CONTOUR
- PROPOSED DIRECTION OF FLOW
- PROPOSED STORM WATER CATCHMENT AREA
- CATCHMENT AREA NUMBER
- RUNOFF COEFFICIENT
- AREA IN HECTARES

108	0.90
0.961	ha

SOURCE:
 TOPOGRAPHIC SURVEY INFORMATION OBTAINED FROM J.D. BARNES LTD.
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 DRAWING FILE: 17-30-157-02-A
 DATED: JULY 2, 2020
 No. 00119541598F ELEVATION=252.480m
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PROJECT
The Paper Mill, 1 Rosetta Street, Georgetown
 1 Rosetta Street, Georgetown
 Township of Halton Hills

PROJECT NO: 125082	SCALE: 1:300
DRAWN BY: K. Park	CHECKED BY: C. Mason
PROJECT MGR: J. Jenkins	APPROVED BY: J. Jenkins

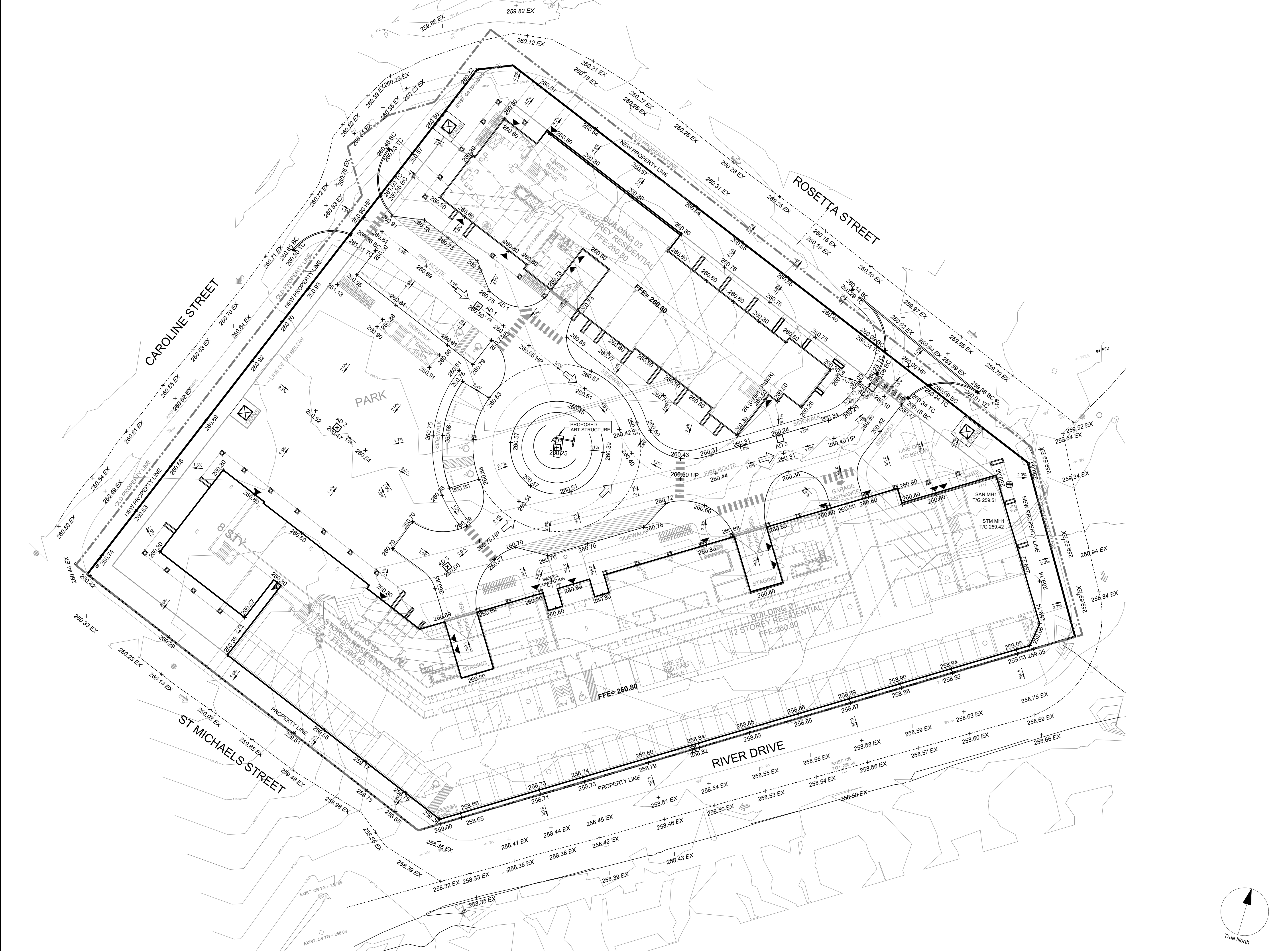
SHEET TITLE
POST-DEVELOPMENT STORM CATCHMENT AREA PLAN

SHEET NUMBER 4	ISSUE 1
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Appendix E – Engineering Plans

Site Grading Plan
Site Servicing Plan

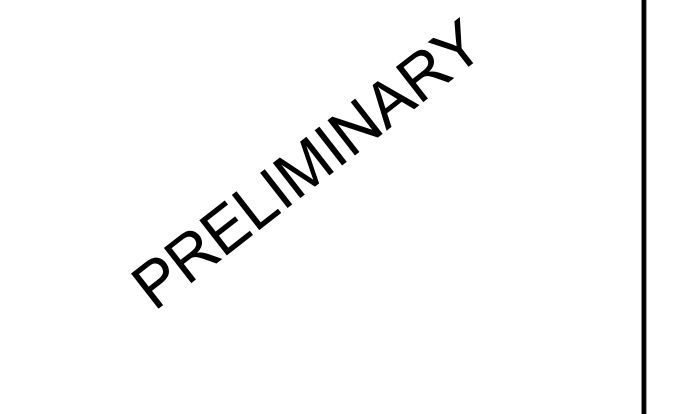


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ISSUES

No.	DESCRIPTION	DATE
1	ISSUED FOR ZONING APPLICATION	MAY 12, 2022



LEGEND

PROPERTY LINE	✕ 149.50
PROPOSED GRADE	✕ 149.33 EX
EXISTING GRADE (TOP OF CURB)	✕ 143.55 TC
PROPOSED GRADE (TOP OF CURB)	✕ 143.55 TC
PROPOSED GRADE (BOTTOM OF CURB)	✕ 149.65 BC
PROPOSED OGS	○
PROPOSED SANITARY MANHOLE	⊗
PROPOSED SINGLE CATCH BASIN	⊠
EXISTING STORM MANHOLE	⊙
EXISTING SANITARY MANHOLE	⊗
EXISTING CATCH BASIN	⊠
PROPOSED VALVE AND BOX	⊕
PROPOSED FIRE HYDRANT	⊕
EXISTING OVERLAND FLOW ROUTE	→
OVERLAND FLOW ROUTE	→

SOURCE:
 TOPOGRAPHIC SURVEY INFORMATION OBTAINED FROM J.D. BARNES LTD.
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 BENCH MARKS: No. 0011954U598F ELEVATION=258.735m



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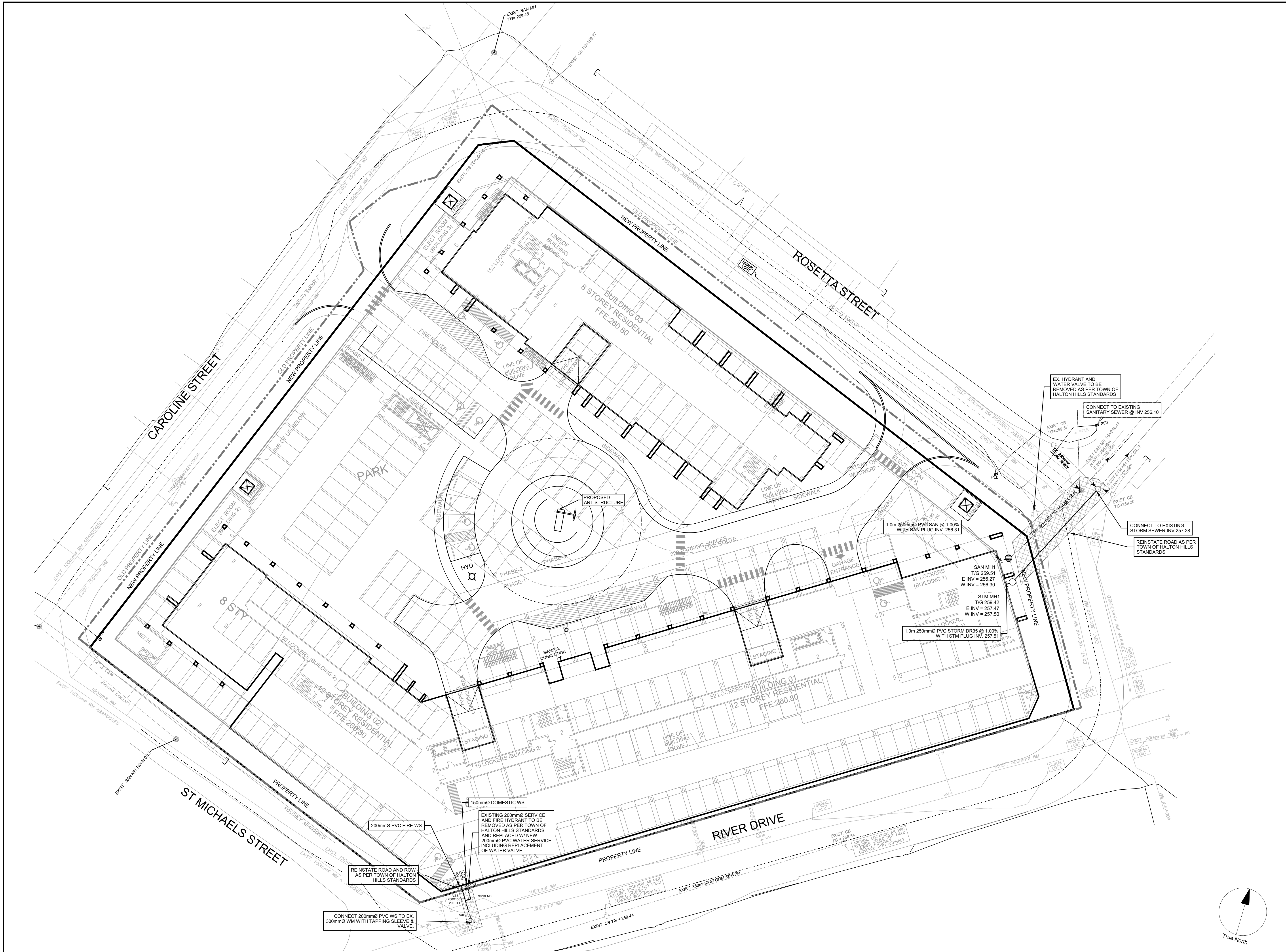
PROJECT
THE PAPER MILL, 1 ROSETTA STREET, GEORGETOWN
 1 ROSETTA STREET, GEORGETOWN
 TOWNSHIP OF HALTON HILLS

PROJECT NO: 125082	SCALE: 1:300
DRAWN BY: K. PARK	CHECKED BY: C. MASON
PROJECT MGR: J. JENKINS	APPROVED BY: J. JENKINS

SHEET TITLE
SITE GRADING PLAN

SHEET NUMBER SG-01	ISSUE 1
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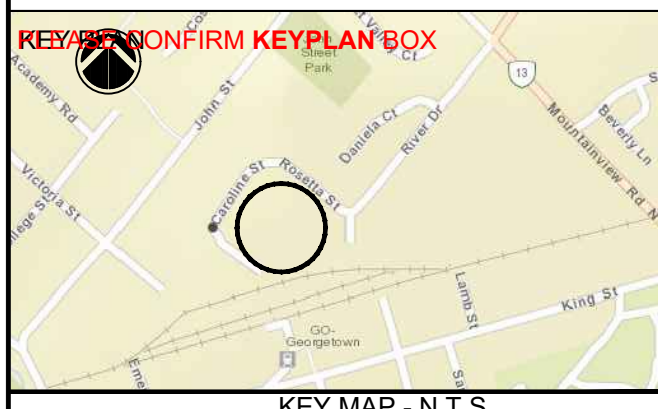
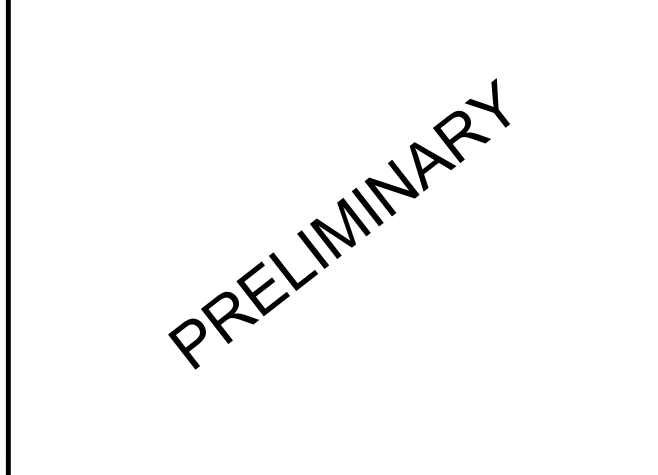


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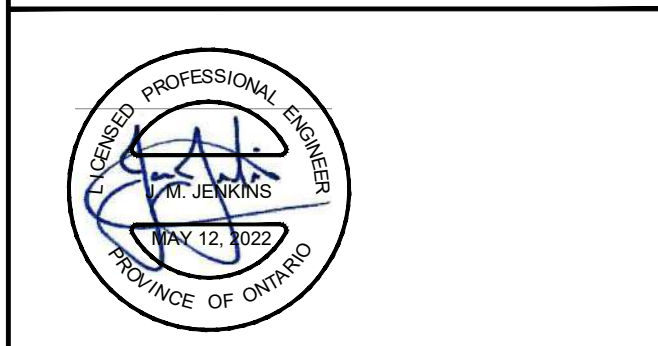
No.	DESCRIPTION	DATE
1	ISSUED FOR ZONING APPLICATION	MAY 12, 2022



LEGEND

PROPERTY LINE	—
PROPOSED DGS	○
PROPOSED SANITARY MANHOLE	○
PROPOSED SINGLE CATCH BASIN	□
EXISTING STORM MANHOLE	○
EXISTING SANITARY MANHOLE	○
EXISTING CATCH BASIN	□
PROPOSED VALVE AND BOX	+
PROPOSED FIRE HYDRANT	+
PROPOSED STORM	—
PROPOSED SANITARY	—
EXISTING STORM	—
EXISTING SANITARY	—
PROPOSED FIRE HYDRANT	+
PROPOSED DOMESTIC WATER METER	[M]
PROPOSED BACKFLOW PREVENTER	[B]
PROPOSED DOUBLE CHECK DETECTOR ASSEMBLY	[D]

SOURCE:
 TOPOGRAPHIC SURVEY INFORMATION OBTAINED FROM J.D. BARNES LTD. MILTON, ON
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PROJECT
THE PAPER MILL, 1 ROSETTA STREET, GEORGETOWN
 1 ROSETTA STREET, GEORGETOWN
 TOWNSHIP OF HALTON HILLS

PROJECT NO: 125082	SCALE: 1:300
DRAWN BY: K. PARK	CHECKED BY: C. MASON
PROJECT MGR: J. JENKINS	APPROVED BY: J. JENKINS

SHEET TITLE
SITE SERVICING PLAN

SHEET NUMBER SS-01	ISSUE 1
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